



MEMORANDUM

April 12, 2005

TO:

J. Kapur/R. Zeldenrust

Bridge and Sturctures Office, MS 47340

FROM:

Tony M. Allen/Mark A. Frye

EEP Geotechnical Branch, MS 47365

SUBJECT:

SR-395, XL 2201

NSLAC – Francis Avenue to US 2 Structures

Bridge Foundation Report

Please find attached the geotechnical report for the SR 395, NSLAC Francis Avenue to US 2 Structures project. This report provides foundation recommendations for the US 395 Northbound Overcrossing at Fairview Road, the US 395 Northbound Overcrossing at Market Street, the US 395 Northbound Overcrossing at Parksmith Drive/BNSF, and the US 395 Undercrossing at Perry Street.

Please contact us if you have any questions or comments regarding the information presented herein.

TMA:maf

Attachment: Geotechnical Report

cc:

Gion Gibson, Eastern Region Materials Engineer

Mohamed Sheikhizadeh, OSC Construction, MS 47354

Mike Gribner, Eastern Region Design Office

GEOTECHNICAL REPORT

SR 395 - NSLAC - Francis Avenue to US 2 Structures

XL-2201

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April 8, 2005



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1. INTRODUCTION

1.1. GENERAL

This report presents the results from a geotechnical study performed for the SR 395, North Spokane Limited Access Corridor (NSLAC), Francis Avenue to US 2 Structures project. The location of the project site is shown on the Vicinity Map, Figure 1 in Appendix A.

The analyses, conclusions, and recommendations provided in this report are based on our understanding of the project and site conditions existing at the time of our site review and field exploration program. The exploratory borings are assumed to be representative of the subsurface conditions at locations throughout the site. If during construction, subsurface conditions differ from those described in the explorations, we should be advised immediately so that we may reevaluate our recommendations and provide assistance.

This report should be made available to prospective bidders and the contractor in accordance with Section 1-02.4 (2) of the Standard Specifications.

1.2. PROJECT DESCRIPTION

The NSLAC project is located in northeast Spokane, between the Spokane River Bridge and the Little Spokane River. The project will entail construction of approximately nine miles of new, four to eight lane highway, up to seven interchanges, and their associated structures. The purpose of the project is to improve mobility through the city of Spokane and Spokane County between Interstate 90 (I-90), US 2, northeastern Washington, and Canada.

The project is divided into a number of sections for the purpose of design and construction. The Francis Avenue to US 2 Structures project will construct five bridges between Gerlach Road and Perry Street. This report provides foundation recommendations for four of those structures, including the US 395 northbound overcrossing at Fairview Road; the US 395 northbound overcrossing at Market Street; the US 395 northbound overcrossing at Parksmith Drive/BNSF; and the US 395 undercrossing at Perry Street. Geotechnical recommendations for the retaining walls necessary to transition from the approach fills to the aforementioned bridges will be provided in a separate report.

1.3. PREVIOUS STUDIES

Information on other geotechnical work performed in the project vicinity is included in the following reports:

- Geotechnical Data Report, SR 395 North Spokane Corridor, Section 1 Hawthorn Road to US 2, Spokane, Washington, prepared by Landau Associates Inc., February 6, 2001.
- Geotechnical Data Report, SR 395 North Spokane Corridor, Section 2 From US 2 to SR 395, prepared by Landau Associates Inc., March 26, 2001.

• Geotechnical Data Report, SR 395 – North Spokane Corridor, Section 3 Spokane, Washington, prepared by Landau Associates Inc., May 23, 2001.

2. PROJECT SUBSURFACE CONDITIONS

2.1. REGIONAL GEOLOGY

Bedrock and sedimentary units in the project area include, from oldest to youngest: Metamorphic and igneous rocks, the Latah Formation and Columbia River Basalt Group, Lake Missoula catastrophic flood deposits, and eolian deposits. These units are described in the following paragraphs.

The Spokane area is underlain by high-grade metamorphic rocks of the Spokane dome of the Priest River metamorphic core complex. The core rocks have been intruded by Cretaceous and early Tertiary granitic rocks. These rocks were deeply eroded, leaving a surface of considerable relief that forms a generally northwest trending mountain range with drainages extending into the lowlands to the south.

During the Miocene Epoch (late Tertiary), between 5 and 24 million years ago, extensive flows of basaltic lava flooded the region, covering the lower valleys and foothills and abutting the higher mountains. The basalt flows covered more than 100,000 square miles in parts of Washington, Idaho, and Oregon. Two formations of the Columbia River Basalt Group have been mapped in the area: the Wanapum Basalt (Priest Rapid Member) and the Grande Ronde Basalt. The Grande Ronde Basalt is between 15.6 and 16.4 million years old, and is generally designated as "valley" flows due mainly to exposures in valleys around the Spokane area. The Wanapum Basalt is between 15 and 14 million years old and is generally designated as "rim rock" flows mainly because it caps the bluffs in the project area.

The Columbia River basin is generally thought to be arcuate in shape, extending from near Grand Coulee, Washington to Moscow, Idaho. Lacustrine sediments, derived from erosion of the older basalts and the pre-Tertiary rocks in the region, were deposited in the basin. These sediments, consisting of primarily silt and clay, with minor sand and gravel, form the Latah Formation. The Latah Formation is generally described as poorly indurated siltstone, claystone, sandstone, and minor conglomerate, containing scattered volcanic ash layers. Little data is available regarding the general depositional nature of the Latah Formation, but sediment was likely deposited by drainages flowing into the basin from the east and north. Where exposed in the Spokane region, the Latah Formation generally overlies the Grande Ronde Basalt and underlies the Wanapum Basalt.

During the Pleistocene Epoch (early Quaternary), 10,000 to 2 million years ago, vast continental ice sheets periodically advanced into the Spokane valley. The latest advance, which occurred between about 12,000 and 22,000 years ago, had the greatest effect on the present day landscape. Melt water deposits, chiefly of sand and gravel, with silt and clay, were deposited in and along the valleys of the Little Spokane and Spokane Rivers. In addition, a proglacial lake, known as Lake Columbia, occupied much of the Spokane Valley during the Pleistocene. Remnants of the lake sediments exist in tributary valleys such as the

Peone Prairie north of Spokane (east of the project area). The glacial lake deposits consist predominantly of sand, silt and clay with scattered drop stones.

Glacial ice of the Purcell lobe is thought to have periodically blocked the Clark Fork River near the present day Idaho/Montana border, forming a great ice dam across the valley. Melt water from other ice lobes further up the Clark Fork River drainage became impounded behind the ice dam, forming a vast lake in present-day western Montana referred to as Glacial Lake Missoula. Periodically the ice dam failed, releasing an enormous volume of water that flowed across the landscape. The majority of this flood water rushed through the Spokane River and Little Spokane River valleys en route to the Columbia River.

Though the number of Pleistocene flood events are unknown, each flood event likely swept down the Spokane and Little Spokane River valleys scouring deposits of the previous flood events, cutting new channels into the pre-Pleistocene bedrock, and leaving behind new deposits of boulders, cobbles, gravel, and sand. In less energetic environments, slack water deposits of chiefly sand and lacustrine sediments were laid down.

Surficial deposits of wind-blown sand (Holocene [present to 10,000 years ago] and Pleistocene Epoch) are present over the flood deposits over much of the project area, south of Farwell Road. The wind-blown deposits (dunes) are derived primarily from Pleistocene flood deposits that mantle much of the project area.

As part of this study, we reviewed available geologic data for the project vicinity. The geologic map of the site, titled *Geologic Map of the Spokane 1:100,000 Quadrangle, Washington-Idaho* (Joseph, 1990), indicates the project area is underlain by wind-blown sand deposits. These deposits generally consist of unconsolidated fine to medium sand.

2.2. FIELD EXPLORATION

Subsurface exploration for this project consisted of advancing one or more exploratory borings at or near the proposed location of each bridge pier. Our field exploration was conducted in general accordance with the 2003, *Checklist and Guidelines for Review of Geotechnical Reports and Preliminary Plans and Specifications*, FHWA publication ED-88-053. Appendix B contains copies of the boring logs and a detailed discussion of our exploration program. All boring logs should be made available to prospective bidders and included in the contract documents. Appendix C provides a discussion of the laboratory testing program and applicable test results. Appendix D contains photographs of rock cores retained during the exploration.

2.3. SOIL CONDITIONS

Based on our exploration program and laboratory testing, there are three general soil units underlying the proposed bridges, which is generally consistent with the available geologic mapping. These soil units are described below in more detail, as well as conditions encountered at each bridge site.

Unit 1 - Dunes: This unit consists of active and stabilized dunes consisting of loose to medium dense poorly graded sand. This unit varied in thickness from a few feet to more than 30 feet.

Unit 2 – Flood Deposits: This unit generally underlies the dune sands and consists of dense to very dense sands and gravels interlayered with fine grained soils. Boulders were encountered in this unit.

Unit 3 – Granite: This unit consists of completely weathered, very weak rock to slightly weathered, strong rock. Where encountered, the rock became less weathered and stronger with depth.

2.3.1. US 395 Northbound Overcrossing at Fairview Road

Soils in the vicinity of the US 395 Northbound Overcrossing at Fairview Road consists of 10 to 15 feet of loose to medium dense sand (Dunes) overlying granite. The granite is completely weathered at the contact with the sand, and becomes less weathered with depth.

A plan view of the bridge, including the location of the exploratory borings, is shown on Figure 2 in Appendix A. A bridge profile and geologic cross-section is shown on Figure 3 in Appendix A.

2.3.2. US 395 Northbound Overcrossing at Market Street

Soils in the vicinity of the US 395 Northbound Overcrossing with Market Street consists of 25 to 35 feet of loose to medium dense sand (Dunes) overlying dense to very dense sands and gravels (Flood Deposits). Completely weathered granite was encountered at a depth of 90 feet in boring MRK-3-04.

A plan view of the bridge, including the location of the exploratory borings, is shown on Figure 4 in Appendix A. A bridge profile and geologic cross-section is shown on Figure 5 in Appendix A.

2.3.3. US 395 Northbound Overcrossing at Parksmith Drive/BNSF

Soils in the vicinity of the US 395 Northbound Overcrossing at Parksmith Drive/BNSF consist of 5 to 10 feet of loose to medium dense sand (Dunes) overlying dense to very dense sand and gravels (Flood Deposits). Granite boulders were encountered within the Flood Deposits at various depths.

A plan view of the bridge, including the location of the exploratory borings, is shown on Figure 6 in Appendix A. A bridge profile and geologic cross-section is shown on Figure 7 in Appendix A.

2.3.4. US 395 Undercrossing at Perry Street

Soils in the vicinity of the US 395 Undercrossing at Perry Street consists of 5 to 15 feet of loose to medium dense sand (Dunes) overlying dense to very dense sands and gravels (Flood Deposits).

A plan view of the bridge, including the location of the exploratory borings, is shown on Figure 8 in Appendix A. A bridge profile and geologic cross-section is shown on Figure 9 in Appendix A.

2.4. GROUNDWATER

Piezometers installed in several of the exploratory borings indicate there is no groundwater table within the soil depths investigated. Several borings show a groundwater table recorded at the time of drilling. We believe this to be drilling fluid trapped in the soil formation and not representative of an actual groundwater table.

3. SEISMOLOGICAL CONSIDERATIONS

3.1. DESIGN EARTHQUAKE PARAMETERS

For seismic design, a peak bedrock acceleration coefficient of 0.06 is recommended based on the June 1996 US Geological Survey National Seismic Hazards Mapping project, as cited in the *Bridge Design Manual*. The recommended acceleration coefficient is based on expected ground motion at the project site that has a 10 percent probability of exceedence in a 50-year period (475-year return period). Design response spectra presented in the AASHTO guide specifications for seismic design of highway bridges are considered appropriate for seismic design of these bridges. A type II Soil Profile response spectrum, with a Site Coefficient of 1.2 is recommended for seismic design.

3.2. SEISMIC HAZARDS

Soil liquefaction is a phenomenon whereby saturated soil deposits temporarily lose strength and behave as a viscous fluid in response to cyclic loading. Soil types considered at the highest risk of liquefaction during a seismic event are loose to medium dense, sandy soils. While other studies have indicated these soils do exist within the project corridor limits, they were not encountered at the structures addressed in this report. Consequently, we do not anticipate any ground settlement or lateral spreading to occur following a design seismic event.

4. BRIDGE FOUNDATION RECOMMENDATIONS

4.1. GENERAL

We understand the bridges associated with this project will be designed using Load and Resistance Factored Design (LRFD) methodology. Based on the soil conditions observed

during our field exploration program, we recommend spread footings be used for all of the bridge foundations. Recommendations for spread footings include nominal resistance for service, strength, and extreme limit states. Lateral earth pressure coefficients and soil parameters for use in designing abutment walls and curtain walls are also provided.

4.2. SPREAD FOOTINGS

Appendix E contains tables of bearing capacity versus footing width for service, strength, and extreme limit states for each bridge. In order to provide adequate service limit state bearing capacity and/or an adequate factor of safety for global stability, a maximum footing elevation is specified for some of the bridge piers. Where unspecified, the footing elevations should be determined by the structural engineer in accordance with Figure 9.5.1-1 of the *Bridge Design Manual*.

Equivalent spring constants for the spread footing foundations should be determined by the method outlined in Section 7.2.4 of FHWA Report No. IP-87-6 titled: *Seismic Design and Retrofit for Highway Bridges*. The shear modulus and Poisson's ratio of the foundation soil must be estimated to calculate the equivalent spring constants using this method. Based on the results of our analysis, we have developed a range of shear modulus values for the soil units under these subject spread footings. The most critical spring constant depends on the rigidity of the superstructure. This is determined by the structural engineer. Ranges of shear modulus values are presented in Appendix E, so as to determine which is more critical, a weak or stiff spring.

We recommend the following resistance factors be used when evaluating the different limit states.

Spread	Footing	Resistance	Factors
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Limit	Resistance Factor φ		
State	Shear Resistance to Sliding	Passive Pressure Resistance to Sliding	Bearing
Strength	0.80	0.50	0.45
Service	N/A	N/A	1.00
Extreme	0.90	0.90	0.90

4.2.1. Service Limit State Settlement

Appendix contains tables for bearing capacity at the Service Limit State. These bearing capacities are settlement-limited values; the amount of settlement is provided in the tables.

Service Limit State settlements at the Fairview Road and Perry Street bridges are based on the customary one inch of settlement. This settlement represents a total settlement at the footing widths and bearing pressures provided in the tables. Differential settlements, both longitudinal and transverse to the structure, are anticipated to be less than ¾ inch.

Service Limit State Capacities for the Market Street bridge have been based on two inches of settlement. Limiting the allowable settlement to one inch would necessitate the use of deep foundations at this bridge. Based on our meetings with the WSDOT Bridge and Structures Office, we understand the proposed structure can tolerate the estimated settlement, and there will be a significant cost savings using spread footings over deep foundations. Differential settlements between the bridge piers are anticipated to be less than 1½ inches. Differential settlements transverse to the piers (along the centerline of bearing) are anticipated to be less than ½ inch over the length of the footings.

Service Limit State Capacities for the Parksmith Drive/BNSF bridge have been based on 1½ inches of settlement. Typically, Service Limit State settlements are limited to less than one inch for multi-span bridges. However, due to the nature of the bridge construction sequence, we understand this bridge will remain settlement tolerant until a final concrete closure pour that will occur after the bridge deck has been poured. We recommend the construction sequence be included in the documents. Differential settlements between the piers are anticipated to be less than 1¼ inches. Differential settlements transverse to the piers (along the centerline of bearing) are anticipated to be less than ½ inch over the length of the footings.

Where indicated in Appendix E, we recommend the near surface soils be excavated and replaced with Gravel Borrow per Standard Specification 9-03.14(1) compacted in accordance with Standard Specification 2-03.3(14) C, Method C. A detail of the excavation and replacement limits is provided on Figure 10 in Appendix A. Elevation limits for the excavation and replacement are provided in Appendix E. Where footing elevations will be above existing ground elevations, we recommend the fill under the footing be placed in accordance with Figure 10.

4.2.2. Settlement Monitoring

We recommend the settlement of the spread footings be monitored during construction. Careful monitoring of footing settlement may allow us to refine our Service Limit State design recommendations for future structures within the US 395 North Spokane Corridor. Sitespecific settlement behavior may result in smaller footings and decreased construction costs.

A survey monument should be placed on the toe of each spread footing immediately after the footing is poured (before construction of the abutment wall). The elevation of the monument should be measured to within 0.01 feet at the completion of each major portion of the bridge construction (footing, abutment wall, girders, bridge deck, and backfill of abutment). A riser pipe may be necessary if the footing is backfilled prior to completion of bridge construction. As several of these bridges will have large approach embankments, we recommend a benchmark be selected that is well away from any construction activity. We are available to assist is developing a special provision for inclusion in the contract plans for this work.

4.3. ABUTMENT WALLS

Abutment walls should be designed using the lateral earth pressure coefficients and soil parameters provided in Appendix E, in conjunction with the design methodology presented in the WSDOT *Bridge Design Manual*. We recommend disregarding the upper 2 feet of soil against the front face of the abutment wall when determining passive soil resistance unless pavement is located at the toe. Per the *Bridge Design Manual*, the lateral earth pressure due to traffic surcharge loading can be calculated using a uniformly distributed load at the ground surface of 250 psf multiplied by K_a .

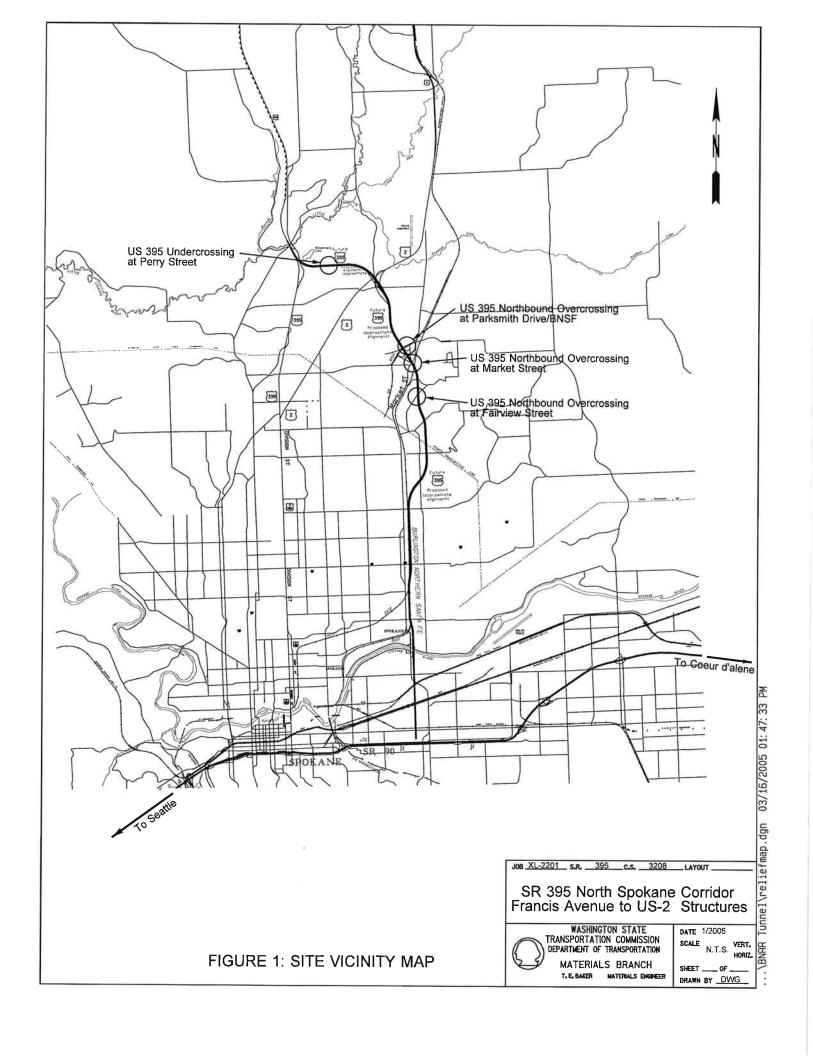
4.4. BRIDGE APPROACH SLABS

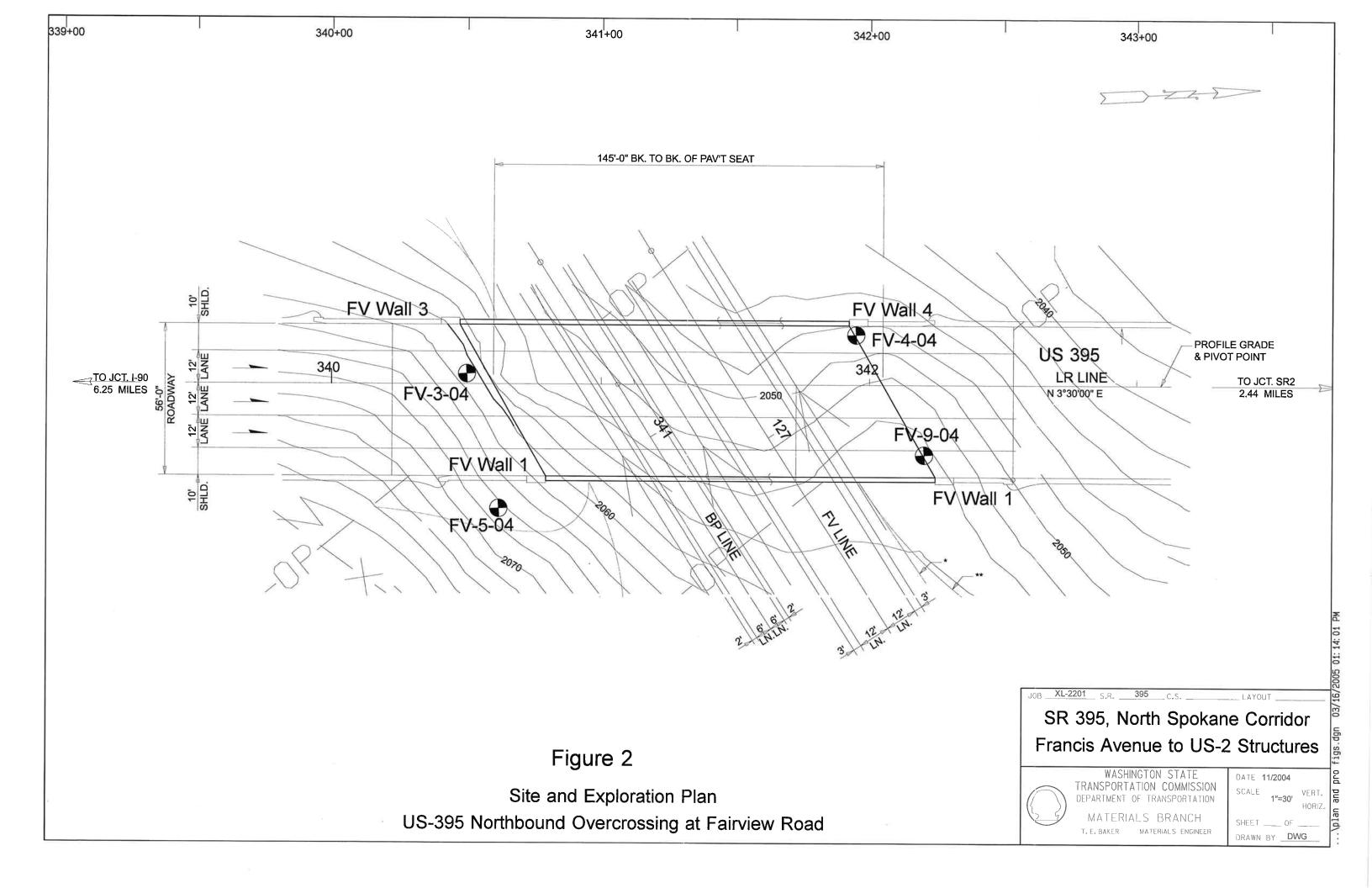
The Design Manual Section 1120.03(6) requires all bridges to have approach slabs unless approval for their deletion has been given. We anticipate there will be some differential settlement between the bridge abutments and the approach embankments. We recommend that approach slabs be used at all of the bridges discussed in this report.

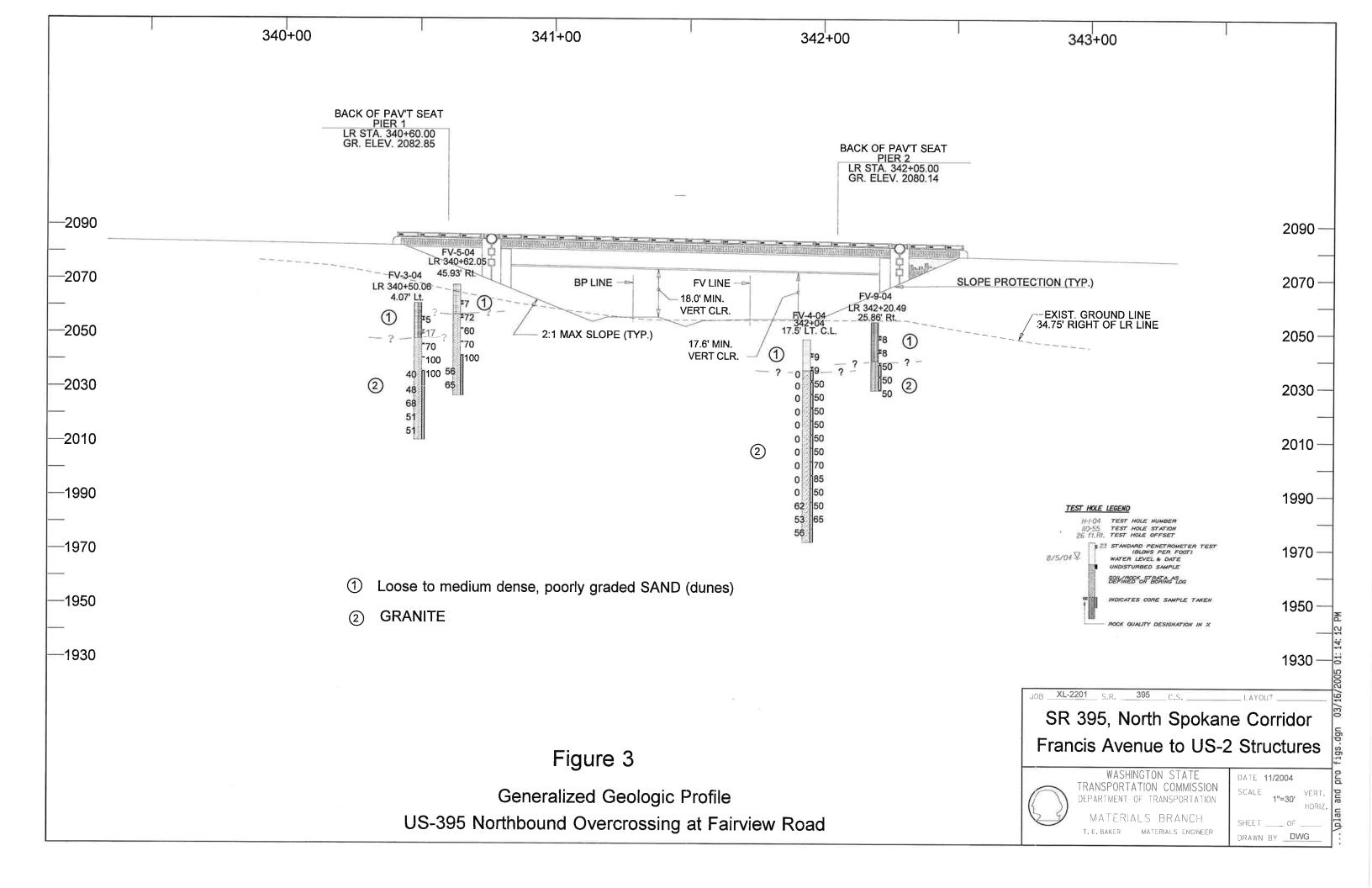
5. CONSTRUCTION CONSIDERATIONS

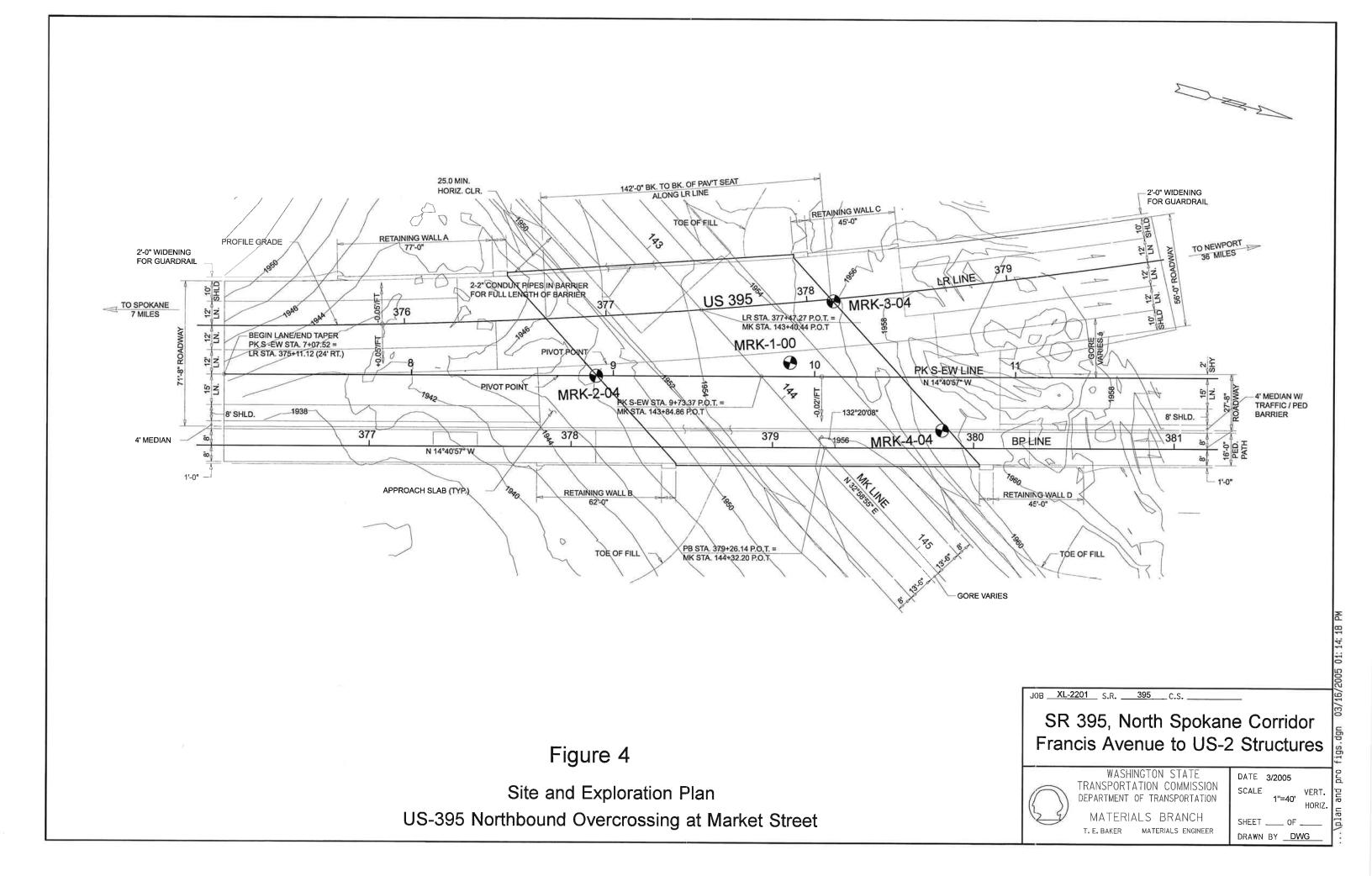
Bedrock will likely be encountered during the excavation for the spread footings at the US 395 Northbound Overcrossing at Fairview Road. Within the limits of the anticipated excavation, the rock will be completely weathered and very weak. We believe the excavation can be achieved with conventional earth moving equipment. Drilling and blasting will likely not be necessary.

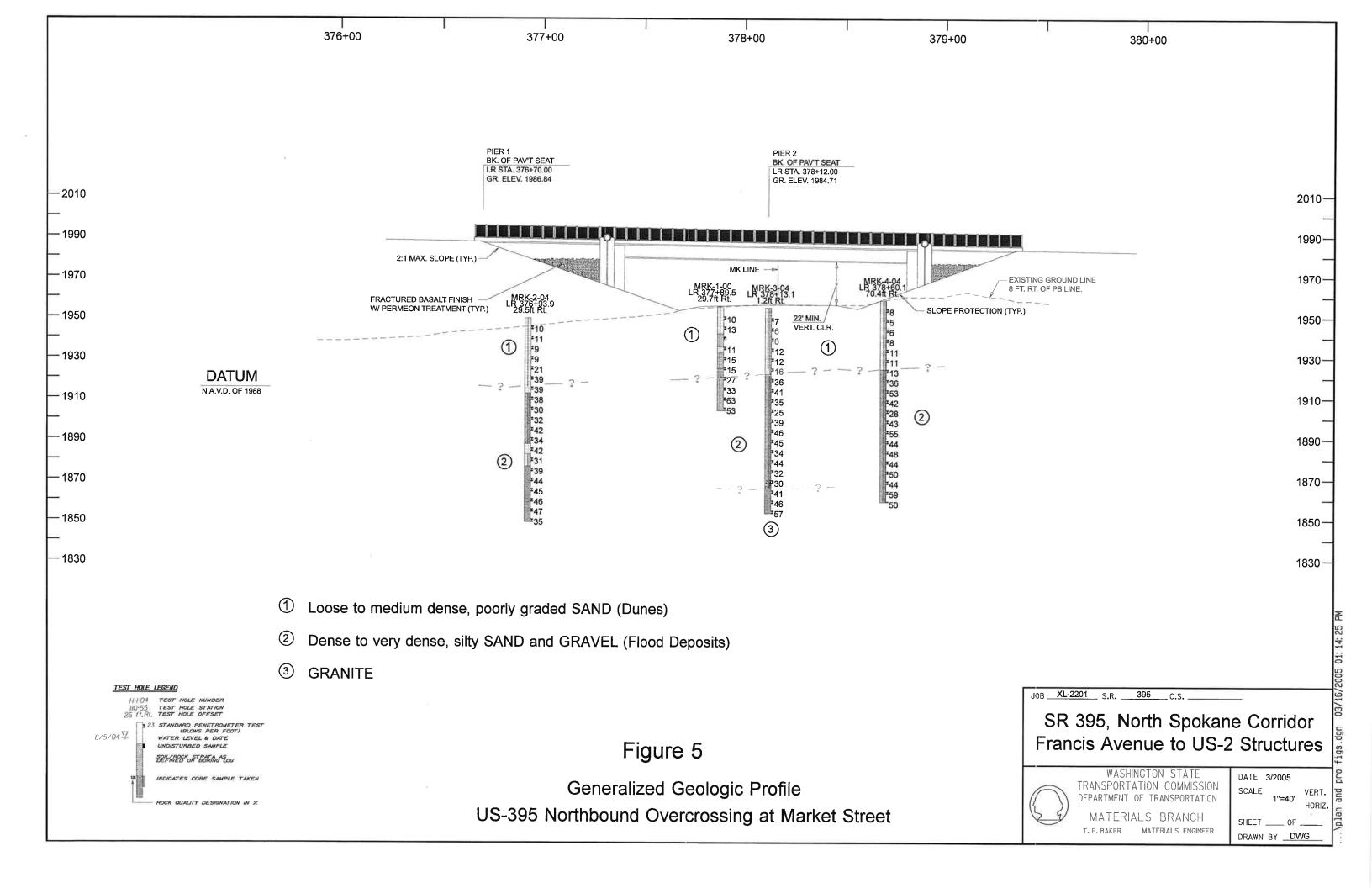
APPENDIX A - FIGURES

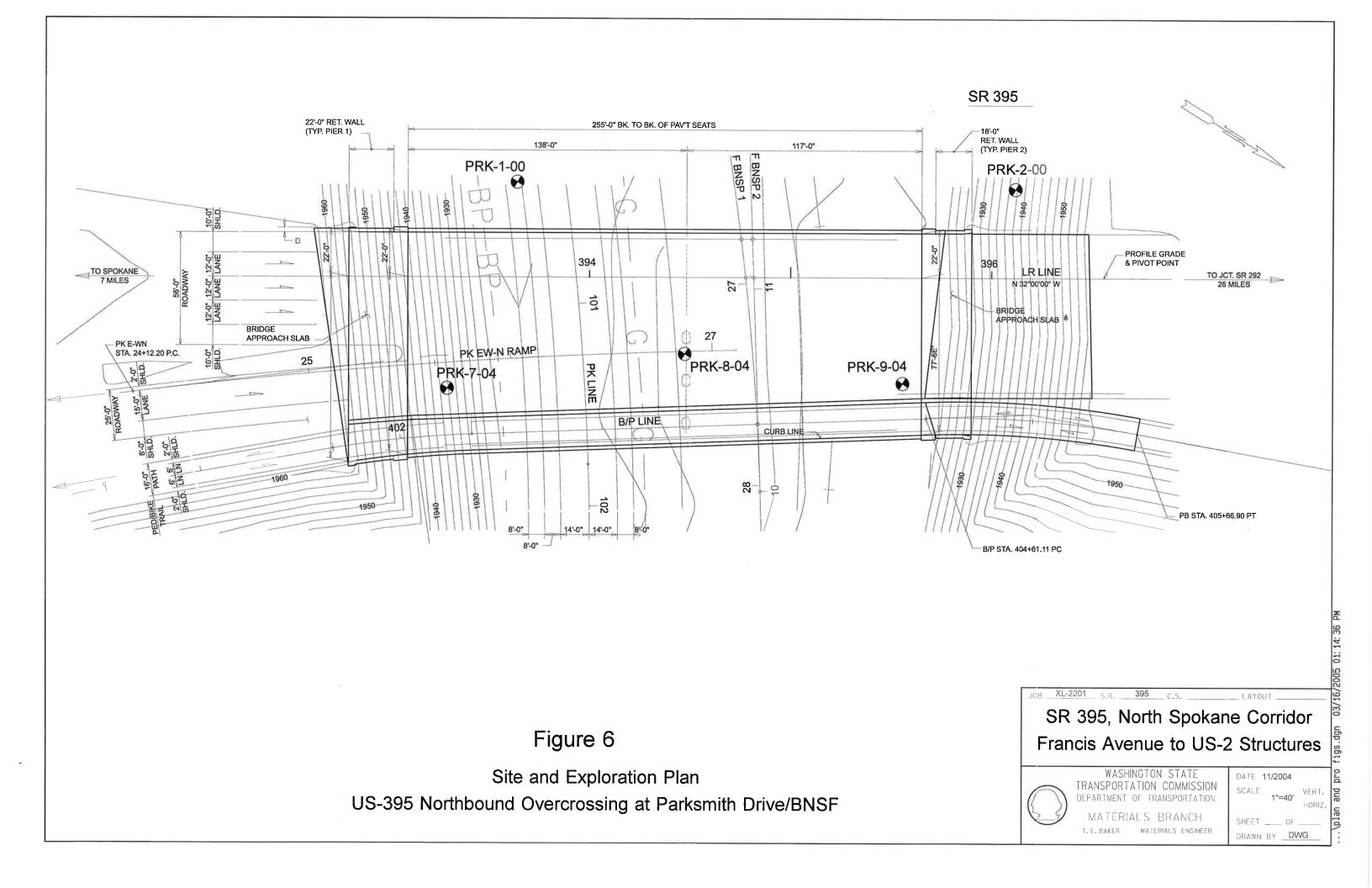


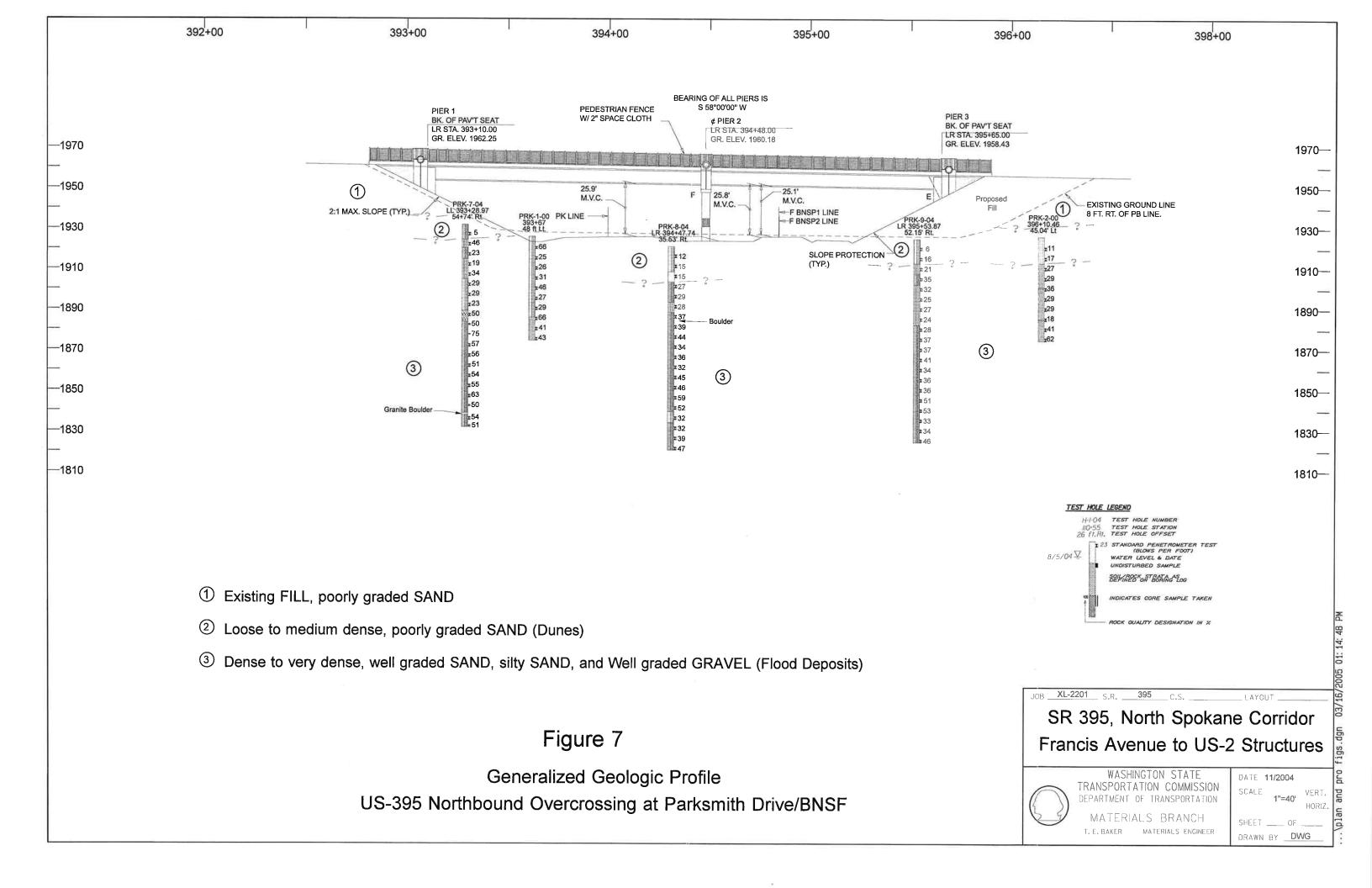


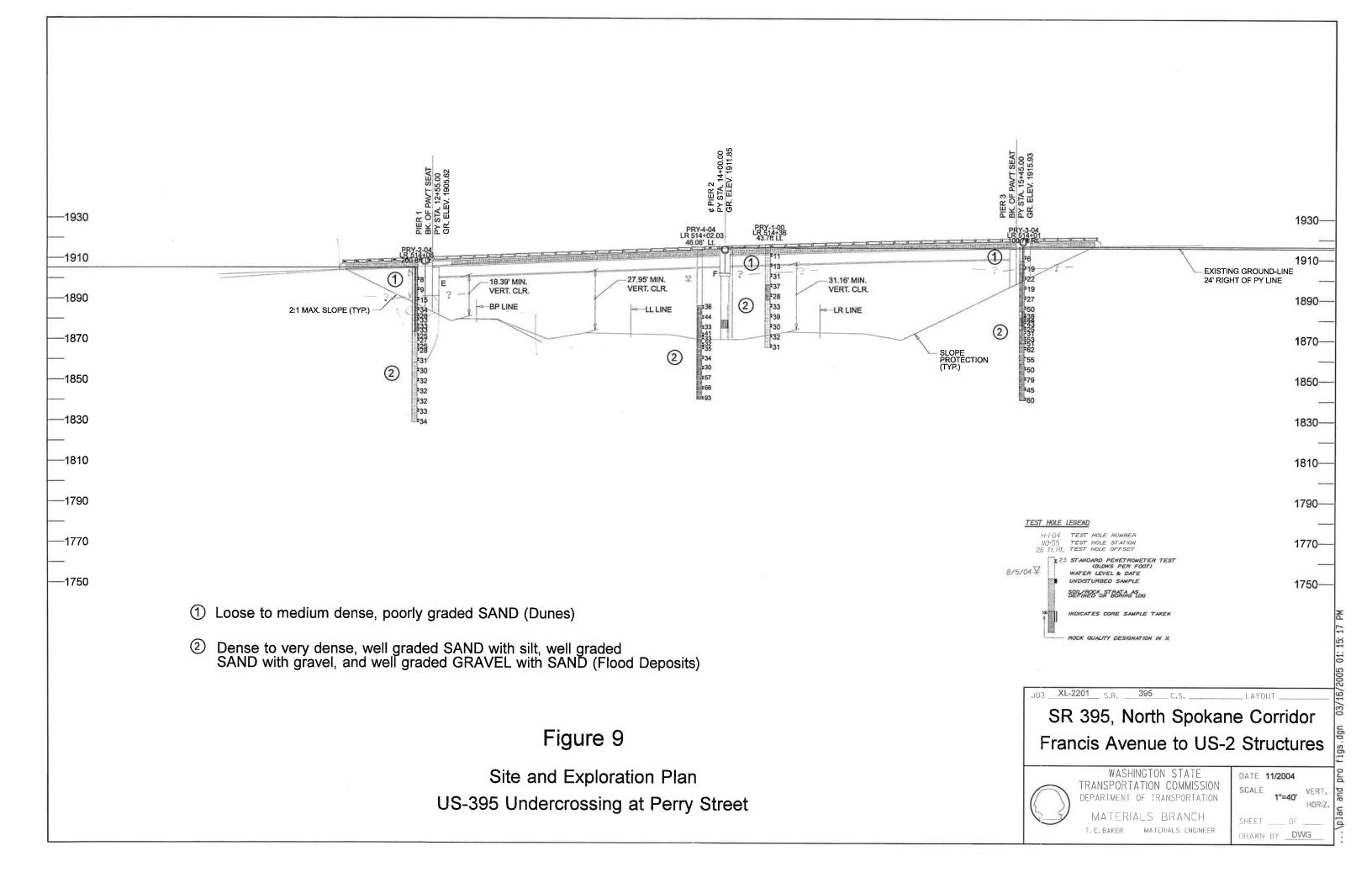


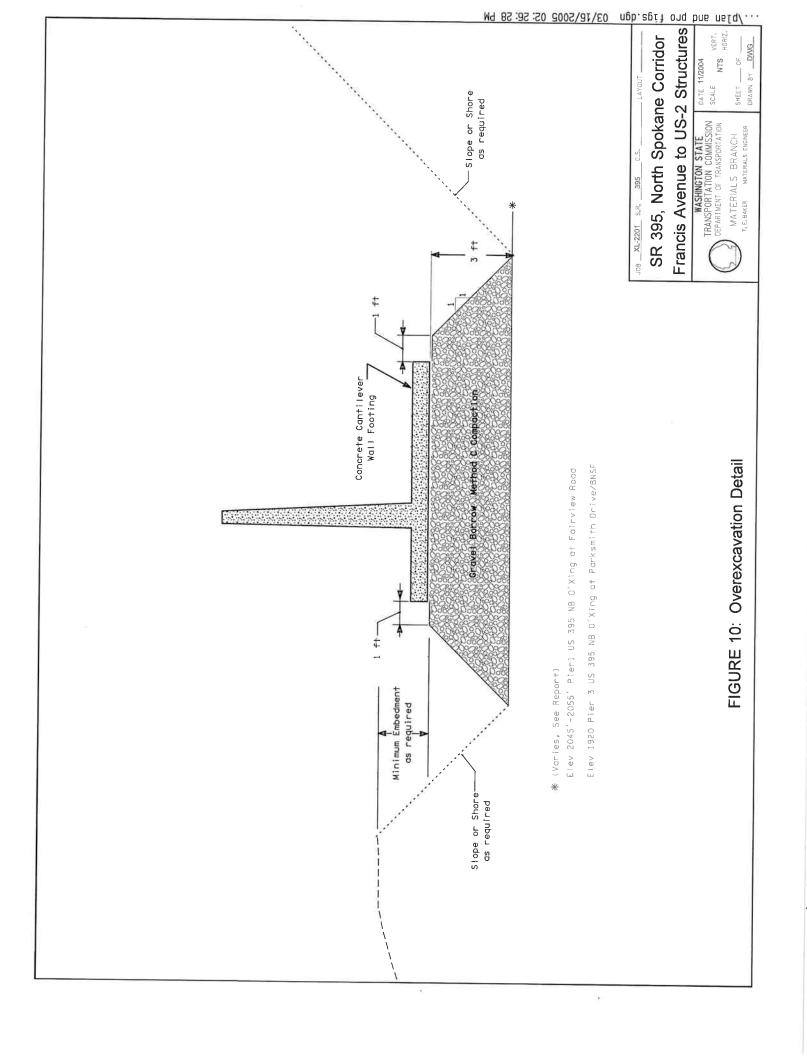












APPENDIX B - FIELD EXPLORATIONS

FIELD EXPLORATIONS

WSDOT's field exploration program for the Francis Avenue to US 2, Structures project consisted of drilling 13 exploratory borings.

Geotechnical drilling was performed using a CME 850 track-mounted drilling rig and a CME-45 skid-mounted drill rig. Test holes were advanced to depths up to 100 feet below the ground surface principally using mud rotary drilling methods. At each location, soil samples were obtained using a SPT (Standard Penetration Test) sampler, in general accordance with ASTM D-1586. SPTs are obtained by driving a 2-inch outside diameter split-spoon sampler 18-inches into the soil with a 140-pound hammer. The number of blows required to achieve each 6 inches of penetration is recorded and the soil's SPT resistance, or N-value, is calculated as the number of blows required to achieve the final 12 inches of penetration. Each drill rig is equipped with an automatic trip hammer to drive the split-spoon sampler. The automatic hammers on these two drill rigs are rated at approximately 80 percent efficiency, as compared to approximately 60 percent for manual hammers.

Select soil samples were then submitted to the E&EP Materials Laboratory for laboratory testing.



Test Boring Legend

Page 1 of 2

	Sampler Symbols
X	Standard Penetration Test
	Oversized Penetration Test (Dames & Moore, California)
	Shelby Tube
P	Piston Sample
	Washington Undisturbed
	Vane Shear Test
	Core
0 0	Becker Hammer
B	Bag Sample

	Well Symbols
	Cement Surface Seal
	Piezometer Pipe in Granular Bentonite Seal
	Piezometer Pipe in Sand
	Well Screen in Sand
X	Granular Bentonite Bottom Seal
	Inclinometer Casing in Concrete Bentonite Grout

	_aboratory Testing Codes	
UU	Unconsolidated Undrained Triaxial	
CŲ	Consolidated Undrained Triaxial	
CD	Consolidated Drained Triaxial	
UC	Unconfined Compression Test	
DS	Direct Shear Test	
CN	Consolidation Test	
GS	Grain Size Distribution	
MC	Moisture Content	
SG	Specific Gravity	
OR	Organic Content	
DN	Density	
AL	Atterberg Limits	
PT	Point Load Compressive Test	
SL	Slake Test	
DG	Degradation	
LA	LA Abrasion	
1		

	Soil Density	/ Modifiers	3
Gravel,	Sand & Non-plastic Silt	Elastic	Silts and Clay
SPT Blows/ft	Density	SPT Blows/ft	Consistency
0-4	Very Loose	0-1	Very Soft
5-10	Loose	2-4	Soft
11-24	Medium Dense	5-8	Medium Stiff
25-50	Dense	9-15	Stiff
>50	Very Dense	16-30	Very Stiff
		31-60	Hard
		>60	Very Hard

A	ngularity of Gravel & Cobbles	
Angular	Coarse particles have sharp edges and relatively plane sides with unpolished surfaces.	
Subangular	Coarse grained particles are similar to angular but have rounded edges.	
Subrounded	Coarse grained particles have nearly plane sides but have well rounded corners and edges.	
Rounded	Coarse grained particles have smoothly curved sides and no edges.	

Soil Moisture Modifiers		
Dry	Absence of moisture; dusty, dry to touch	
Moist	Damp but no visible water	
Wet	Visible free water	

	Soil Structure
Stratified	Alternating layers of varying material or color at least 6mm thick; note thickness and inclination.
Laminated	Alternating layers of varying material or color less than 6mm thick; note thickness and inclination.
Fissured	Breaks along definite planes of fracture with little resistance to fracturing.
Slickensided	Fracture planes appear polished or glossy, somtimes striated.
Blocky	Cohesive soil that can be broken down into smaller angular lumps which resist further breakdown.
Disrupted	Soil structure is broken and mixed. Infers that material has moved substantially - landslide debris.
Homogeneous	Same color and appearance throughout.

	HCL Reaction
No HCL Reaction	No visible reaction.
Weak HCL Reaction	Some reaction with bubbles forming slowly.
Strong HCL Reaction	Violent reaction with bubbles forming imediately.

Degree of Vesicularity of Pyroclastic Rocks		
Slightly Vesicular	5 to 10 percent of total	
Moderately Vesicular	10 to 25 percent of total	
Highly Vesicular	25 to 50 percent of total	
Scoriaceous	Greater than 50 percent of total	

Test Boring Legend

Page 2 of 2

		Grain Size
Fine Grained	< 1mm	Few crystal boundaries/grains are distinguishable in the field or with hand lens.
Medium Grained	1mm to 5mm	Most crystal boundaries/grains are distinguishable with the aid of a hand lens.
Coarse Grained	> 5mm	Most crystal boundaries/grains are distinguishable with the naked eye.

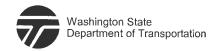
Ξ.	Weathered State	
Term	Description	Grade
Fresh	No visible sign of rock material weathering; perhaps slight discoloration in major discontinuity surfaces.	I
Slightly Weathered	Discoloration indicates weathering of rock material and discontinuity surfaces. All the rock material may be discolored by weathering and may be somewhat weaker externally than its fresh condition.	П
Moderately Weathered	Less than half of the rock material is decomposed and/or disintegrated to soil. Fresh or discolored rock is present either as a continuous framework or as core stones.	ın
Highly Weathered	More than half of the rock material is decomposed and/or disintegrated to soil. Fresh or discolored rock is present either as discontinuous framework or as core stone.	IV
Completely Weathered	All rock material is decomposed and/or disintegrated to soil. The original mass structure is still largely intact.	V
Residual Soil	All rock material is converted to soil. The mass structure and material fabric is destroyed. There is a large change in volume, but the soil has not been significantly transported.	VI

- : a	8 L T	Relative Rock Strength	
Grade	Description	Field Identification	Uniaxial Compressive Strength approx
R1	Very Weak	Specimen crumbles under sharp blow from point of geological hammer, and can be cut with a pocket knife.	1 to 25 MPa
R2	Moderately Weak	Shallow cuts or scrapes can be made in a specimen with a pocket knife. Geological hammer point indents deeply with firm blow.	25 to 50 MPa
R3	Moderately Strong	Specimen cannot be scraped or cut with a pocket knife, shallow indentation can be made under firm blows from a hammer.	50 to 100 MPa
R4	Strong	Specimen breaks with one firm blow from the hammer end of a geological hammer.	100 to 200 MPa
R5	Very Strong	Specimen requires many blows of a geological hammer to break intact sample.	Greater than 200 MPa

Discontinuities

S	pacing -		Condition
Very Widely	Greater than 3 m	Excellent	Very rough surfaces, no separation, hard discontinuity wall
Widely	1 m to 3 m	Good	Slightly rough surfaces, separation less than 1 mm, hard
Moderately	0.3 m to 1 m		discontinuity wall.
Closely	50 mm to 300 mm	Fair	Slightly rough surfaces, separation greater than 1 mm, soft discontinuity wall.
Very Closely	Less than 50 mm		
RO	QD (%)	Poor	Slickensided surfaces, or soft gouge less than 5 mm thick, or open discontinuities 1 to 5 mm.
	th of core run	Very Poor	Soft gouge greater than 5 mm thick, or open discontinuities greater than 5 mm.

Fracture Frequency (FF) is the average number of fractures per 300 mm of core. Does not include mechanical breaks caused by drilling or handling.



Elevation __2062.0 ft (628.5 m)

Start Card S 23721

HOLE No. FV-3-04

Inspector Cleo Andrews

Sheet __1__ of __3__

Driller Kerry Cooper Lic# 2552

Project NSLAC - Perry, Market and Fairview Structures

Site Address Vicinity of Fairview Road and Prososed SR 395

Start September 22, 2004 Completion September 23, 2004 Well ID# Equipment CME 55 w/ autohammer

Station LR 340+50.06

SR 395__

Offset 4.07' Lt. Casing HQ 3" ID x 80.0'

Method Wet Rotary

Northing 618259.786

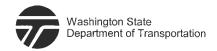
Job No. XL-2201

Easting _ 2823018.831

Latitude

_____Longitude_

Depth (ft)	Meters (m)	Profile	Standard Penetration Blows/ft 10 20 30	SPT Blows/6 (N)	Sample Type	Sample No. (Tube No.)	Lab Tests	Description of Material	Groundwater	Instrument
5—	-2			2 2 3 (5)	X	D-1		Poorly graded SAND, with 0.4' of sandy lean clay, loose, olive gray, moist, Stratified. 0.0' to 5.0' silty sand as shown on surface and by drilling fluid returns. 100% drilling fluid return. Length Recovered 1.5 ft, Length Retained 1.0 ft Poorly graded SAND, with silty Sand, medium dense, brown, moist, Stratified. Length Recovered 1.5 ft, Length Retained 1.0 ft		
15—	-4 -5	ニケーケーケーケーケーケーケーケーケーケーケーケーケーケーケーケーケーケーケー		70/5 (70)	×	D-3		GRANITE, pale brown, medium grained, completely weathered, very weak rock. Length Recovered 0.4 ft, Length Retained 0.4 ft		



Start Card S 23721

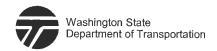
HOLE No. FV-3-04

Sheet 2 of 3

Job No_XL-2201______ SR

395 Elevation 2062.0 ft (628.5 m)

Project NSLAC - Perry, Market and Fairview Structures Driller Kerry Cooper Lic# 2552 Groundwater Standard Sample No. \equiv SPT (Tube No.) Instrument Depth (ft) Tests Penetration Meters Lab Blows/6' Description of Material Blows/ft (N) 10 30 D-4 100/4 GRANITE, pale brown, medium grained, completely (100)weathered, very weak rock. Length Recovered 0.3 ft, Length Retained 0.3 ft 25 100/4 D-5 GRANITE, light yellowish brown, medium grained, highly C-6 (100)weathered, very weak rock. Discontinuities are very widely spaced and in very poor condition, (Changed to RQD 40 more in tact rock at 23.3). FF Length Recovered 0.3 ft, Length Retained 0.3 ft 6 GRANITE, moderately fractured, light greenish gray, medium grained, slightly weathered, very strong rock. Discontinuities are moderately spaced and in fair condition, Percent Recovered 80.0% 30 RQD C-7 GRANITE, moderately fractured, light greenish gray, 48 medium grained, slightly weathered, very strong rock. FF Discontinuities are moderately spaced and in fair ALL XL-2201 NSLAC - PERRY, MARKET AND FARVIEW STRUCTURES.GPJ SOIL, GDT 2/11/05,11:30:37 condition, Percent Recovered 100.0% 35 RQD C-8 GRANITE, moderately fractured, light greenish gray, 68 medium grained, slightly weathered, very strong rock. FF Discontinuities are moderately spaced and in fair 6 condition, 100% drilling fluid loss starting at 43.0', Percent Recovered 100.0% 12 40 RQD C-9 GRANITE, light greenish gray, medium grained, slightly 51 weathered, very strong rock. Discontinuities are FF moderately spaced and in fair condition, Percent 6 Recovered 100.0% 13



Start Card S 23721

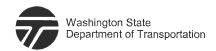
HOLE No. FV-3-04

Sheet <u>3</u> of <u>3</u>

Job No. XL-2201

SR 395 Elevation 2062.0 ft (628.5 m)

	Project	NSLAC	- Perry,	Market	t and I	airview	Structure	s				Driller Kerry Cooper	Lic#_2	2552
Depth (ft)	Meters (m)	Profile	10	Stand Penetr Blow	ation	40	SPT Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab	Tests	Description of Material	Groundwater	Instrument
	-14	1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-	1				RQD 51 FF 5		C-10			GRANITE, light greenish gray, medium grained, slightly weathered, very strong rock. Discontinuities are moderately spaced and in fair condition, Percent Recovered 100.0%		
50 —	— 15	\\\-\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\			 								-	
	— 16] 	 	 							End of test hole boring at 50.5 ft below ground elevation. This is a summary Log of Test Boring. Soil/Rock descriptions are derived from visual field identifications and laboratory test data.		
55 —			1	 								(Test hole dry). Ended and abandoned test boring at 50.5' below ground elevation. 9/23/04.,		
	—17 -		1											
60 —	—18		1 1 1 1 1 1											
-	—19													
65 —	20				1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1									
	- 21				1 1 1 1 1 1 1 1 1									



Elevation 2048.6 ft (624.4 m)

Start Card S 23721

HOLE No. FV-4-04

Sheet __1__ of __4__

Driller Kerry Cooper Lic# 2552

Project NSLAC - Perry, Market and Fairview Structures

SR

Inspector Cleo Andrews

Site Address Vicinity of Fairview Road and Proposed SR-395

Start September 24, 2004 Completion September 28, 2004 Well ID# Equipment CME 45 w/ autohammer

Station LR 341+95.15

Offset 18.54' Lt. Casing HQ 3" ID x 80.0'

Method Wet Rotary

Northing 618405.482

Job No. XL-2201

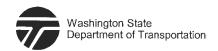
Easting 2823013.244 Latitude

395

____Longitude ___

County Spokane Subsection SE 1/4 of the SW 1/4 Section 15 Range 43 EWM Township 26

Depth (ft) Meters (m)	Profile	Standard Penetration Blows/ft 10 20 30 40	SPT Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab Tests	Description of Material	Groundwater	Instument
5-			2 4 5 (9)	X	D-1	GS MC	SP, M.C. = 22% Poorly graded SAND, loose, brown, moist, Homogeneous. 0.0' to 5.0' Poorly graded Sand as shown in wash return. 100% drilling fluid return. Length Recovered 1.5 ft, Length Retained 1.0 ft		
10 - 3	- W - W - W - W - W - W - W - W - W - W		3 4 5 (9) RQD 0 FF 26	X	D-2 C-3	GS MC	ML, M.C. = 16% Sandy SILT, loose, brown, moist, Homogeneous. Length Recovered 1.5 ft, Length Retained 1.0 ft GRANITE, yellowish orange, medium grained, completely weathered, very weak rock. Discontinuities are very widely spaced and in very poor condition, Percent Recovered 71.0%		
15-5			41 50/4 (50) RQD 0 FF 26	X	D-4 C-5	GS MC	SM, M.C. = 15%, PI = NP GRANITE, yellowish orange, medium grained, completely weathered, very weak rock. Discontinuities are very widely spaced and in very poor condition Length Recovered 0.8 ft, Length Retained 0.8 ft GRANITE, yellowish orange, medium grained, completely weathered, very weak rock. Discontinuities are very widely spaced and in very poor condition, Percent Recovered 68.0%		



Start Card S 23721

HOLE No. FV-4-04

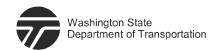
Sheet 2 of 4

Job No. XL-2201

395

Elevation 2048.6 ft (624.4 m)

Depth (ft) Meters (m)	Profile	Standard Penetration Blows/ft 10 20 30	SPT Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab Tests	Description of Material	Groundwater	
-7	100000000000000000000000000000000000000		50/5 (50) RQD 0 FF 26	Ň	D-6 C-7		GRANITE, yellowish orange, medium grained, completely weathered, very weak rock. Discontinuities are very widely spaced and in very poor condition Length Recovered 0.4 ft, Length Retained 0.4 ft GRANITE, yellowish orange, medium grained, completely weathered, very weak rock. Discontinuities are very widely spaced and in very poor condition, Percent Recovered 88.0%	-	
25-	ラング・グ・グ・グ・グ・グ・グ・グ・グ・グ・グ・グ・グ・グ・グ・グ・グ・グ・グ・		50/5 (50) RQD 0 FF 26	Ď	D-8 C-9		GRANITE, yellowish orange, medium grained, completely weathered, very weak rock. Discontinuities are very widely spaced and in very poor condition Length Recovered 0.4 ft, Length Retained 0.4 ft GRANITE, yellowish orange, medium grained, completely weathered, very weak rock. Discontinuities are very widely spaced and in very poor condition, Percent Recovered 85.0%		
-9 30- -10	シー・シー ケーケーケーケーケーケーケーケーケーケーケーケーケーケーケーケーケーケーケ		50/3 (50) RQD 0 FF 26	Ì	D-10 C-11		GRANITE, yellowish orange, medium grained, completely weathered, very weak rock. Discontinuities are very widely spaced and in very poor condition Length Recovered 0.2 ft, Length Retained 0.2 ft GRANITE, yellowish orange, medium grained, completely weathered, very weak rock. Discontinuities are very widely spaced and in very poor condition, Percent Recovered 42.0%		
35-	0-0-0-0-0-0		\$50/5 (50) RQD 0 FF 26	Ď	D-12 C-13		GRANITE, yellowish orange, medium grained, completely weathered, very weak rock. Discontinuities are very widely spaced and in very poor condition Length Recovered 0.3 ft, Length Retained 0.3 ft GRANITE, yellowish orange, medium grained, completely weathered, very weak rock. Discontinuities are closely spaced and in very poor condition, Percent Recovered 99.0%		
-12 40- -13	としているとうというにかいかいかいかいかいかいかいかいかいかいかいかいかいかいかいかいかいかいか		50/3 (50) RQD 0 FF		D-14 C-15		GRANITE, yellowish orange, medium grained, completely weathered, very weak rock. Discontinuities are closely spaced and in very poor condition Length Recovered 0.2 ft, Length Retained 0.2 ft GRANITE, yellowish orange, medium grained, completely weathered, very weak rock. Discontinuities are very closely spaced and in very poor condition, Percent Recovered 100.0%	_	



Start Card S 23721

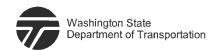
HOLE No. FV-4-04

Sheet <u>3</u> of <u>4</u>

Job No. XL-2201

SR 395 Elevation 2048.6 ft (624.4 m)

Depth (ft)	Meters (m)	Profile	Standard Penetration Blows/ft 10 20 30 40	SPT Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab Tests	Description of Material	Groundwater	
	—14 -	シャークークーク		70/3 (70) RQD 0 FF 26	Ì	D-16 C-17		GRANITE, yellowish orange, medium grained, completely weathered, very weak rock. Discontinuities are very closely spaced and in very poor condition Length Recovered 0.2 ft, Length Retained 0.2 ft GRANITE, yellowish orange, medium grained, completely weathered, very weak rock. Discontinuities are very closely spaced and in very poor condition, Percent Recovered 100.0%	-	
50	—15 —16			85/5 (85) RQD 0 FF 26	X	D-18 C-19		GRANITE, yellowish orange, medium grained, completely weathered, very weak rock, HCI reaction not tested. Discontinuities are very closely spaced and in very poor condition Length Recovered 0.4 ft, Length Retained 0.4 ft GRANITE, yellowish orange, medium grained, completely weathered, very weak rock. Discontinuities are very widely spaced and in very poor condition, Percent Recovered 79.0%	-	
55 —	— 17 -			50/2 (50) RQD 0 FF 26		D-20 C-21		GRANITE, yellowish orange, medium grained, completely weathered, very weak rock. Discontinuities are very closely spaced and in very poor condition Length Recovered 0.1 ft, Length Retained 0.1 ft GRANITE, yellowish orange, medium grained, completely weathered, very weak rock. Discontinuities are very closely spaced and in very poor condition, Percent Recovered 100.0%	_	
60	-18 -19			50/2 (50) RQD 62 FF 5)	D-22 C-23		GRANITE, yellowish orange, medium grained, completely weathered, very weak rock, HCl reaction not tested. Discontinuities are very closely spaced and in very poor condition Length Recovered 0.1 ft, Length Retained 0.1 ft GRANITE, light greenish gray, medium grained, moderately weathered, moderately weak rock. Discontinuities are very closely spaced and in poor condition, Percent Recovered 100.0%		
65—	-20	シーケーケーケーケー	>>	65/2 (65) RQD 53 FF 6		D-24 C-25		No Recovery Basalt , medium dark gray, fine grained, slightly weathered, strong rock. Discontinuities are closely spaced and in fair condition, changed GRANITE at 2.9', Percent Recovered 100.0%		
_	-21									



Start Card S 23721

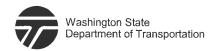
HOLE No. FV-4-04

Sheet <u>4</u> of <u>4</u>

Job No_XL-2201

SR 395 Elevation 2048.6 ft (624.4 m)

	Project	NSLAC	- Perry,	Market	t and I	airview	Structure	s	16			Sneet 4 of 4 Driller Kerry Cooper	Lic#2	2552
Depth (ft)	Meters (m)	Profile	10	Stand Penetr Blow	ation	40	SPT Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab	Tests	Description of Material	Groundwater	Instrument
1	- 22	(RQD 56 FF 7		C-26			GRANITE with iron mineral infilling, light greenish gray, fine grained, slightly weathered, very strong rock. Discontinuities are closely spaced and in fair condition, Percent Recovered 100.0%		
75 —	-23	1.7		 	-								_	
2				 	i ! !	i L L I						End of test hole boring at 75 ft below ground elevation. This is a summary Log of Test Boring. Soil/Rock descriptions are derived from visual field identifications and laboratory test data.	-	
90	— 24		1	 								Bailed hole to dry, no recharge. Ended and abandoned test boring at 75.0' below ground elevation. Dry hole. 9/28/04.	-	
80 —	.] 	1	1							-	
	25			i 1 1									-	
85 —	— 26			1									_	
-	-			 	1								-	
-	— 27			1	1								-	
90 —	_		1	1										
	— 28		1		1									
-	•		i ! !	i	1									



Job No. XL-2201

LOG OF TEST BORING

395 Elevation 2067.6 ft (630.2 m)

Start Card S 23721

HOLE No. _FV-5-04_

Sheet __1 __ of __2

Project NSLAC - Perry, Market and Fairview Structures

Driller Kerry Cooper Lic# 2552

Site Address Vicinity of fairview Road and Proposed SR-395

Inspector Cleo Andrews

Start September 29, 2004 Completion September 29, 2004 Well ID#

SR _

Equipment CME 45 w/ autohammer

Station LR 340+62.05

Offset 45.93' Rt. Casing HQ 3" ID x 45.0'

Method Wet Rotary

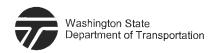
Northing 618268.7

Easting 2823069.463

_Latitude _

__ Longitude _

Depth (ft)	Meters (m)	Profile	10	Standa Penetra Blows	ation s/ft	40	SPT Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab Tests	Description of Material	Groundwater	Instrument
5-	-2		5	2	3		2 4 3 (7)	X	D-1		Poorly graded SAND, loose, brown, moist, Homogeneous. 0.0' to 6.0' Poorly graded Sand as indicated by drilling and wash return. 100% drilling fluid return. Length Recovered 1.5 ft, Length Retained 1.0 ft		
3	4	(ハーハーハーハーハーハーハークークークークークークークークークークークークーク				>>4	20 27 45 (72)	X	D-2		GRANITE, light yellowish orange, medium grained, completely weathered, very weak rock. Length Recovered 1.5 ft, Length Retained 1.0 ft		
15 —	-5					>>4	60/6 (60)	X	D-3		GRANITE, yellowish orange, medium grained, completely weathered, very weak rock. Discontinuities are very widely spaced and in very poor condition Length Recovered 0.5 ft, Length Retained 0.5 ft		



Start Card S 23721

HOLE No. FV-5-04

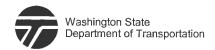
Sheet __2 of __2

Job No_XL-2201

395___ SR

Elevation 2067.6 ft (630.2 m)

Depth (fl)	Meters (m)	Profile	Standard Penetration Blows/ft	40	SPT Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab	Tests	Description of Material	Groundwater
	-7	になったい		>>4	70/4 (70)	×	D-4			GRANITE, yellowish orange, medium grained, completely weathered, very weak rock. Discontinuities are very widely spaced and in very poor condition Length Recovered 0.3 ft, Length Retained 0.3 ft	
25	_8 	からからない かんかん		>><	100/4 (100) RQD 0.125 FF 24	Ň	D-5 C-6			GRANITE, brownish gray, medium grained, completely weathered, very weak rock. Discontinuities are very widely spaced and in very poor condition Length Recovered 0.3 ft, Length Retained 0.3 ft GRANITE with iron minerial infillings, moderately fractured, light greenish gray, medium grained, highly weathered, very weak rock. Discontinuities are closely spaced and in very poor condition, changed to more in tact rock at 28.0', Percent Recovered 100.0%	
	- 10	行うないので		 - -	RQD 56 FF 5		C-7			GRANITE, moderately fractured, light greenish gray, medium grained, slightly weathered, very strong rock. Discontinuities are closely spaced and in fair condition, Percent Recovered 100.0%	
35—	—11 —12	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1			RQD 65 FF 8		C-8			GRANITE with iron minerial infillings, moderately fractured, light greenish gray, medium grained, slightly weathered, very strong rock. Discontinuities are closely spaced and in fair condition, (Note 100% drilling fluid loss starting at 31.0'). Percent Recovered 100.0%	
	– 13									End of test hole boring at 41 ft below ground elevation. This is a summary Log of Test Boring. Soil/Rock descriptions are derived from visual field identifications and laboratory test data.	



Site Address Fairview & Proposed SR 395

LOG OF TEST BORING

Elevation 1996.2 ft (608.4 m)

Start Card S-23735

HOLE No.	FV-9-04

Sheet __1__ of __2_

Inspector Dave Nelson

Driller Sean Verlo Lic# 2615

Project NSLAC - Perry, Market and Fairview Structures

____ SR __395___

Start December 15, 2004 Completion December 16, 2004 Well ID# Equipment CME 45 w/ autohammer

Station LR 342+20.49 Offset 25.86' Rt. Casing HQ x 25.0

Job No. XL-2201

Method Wet Rotary

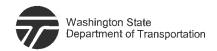
Northing _

_ Easting _ __ Latitude _

Longitude

County Spokane SW 1/4 of Se 1/4

Depth (ft)	Meters (m)	Profile	Standard Penetration Blows/ft 10 20 30	40	SPT Blows/6** (N)	Sample Type	Sample No. (Tube No.)	Lab	Tests	Description of Material	Groundwater	Instrument
	-1											
5	-2				2 4 4 (8)	X	D-1			Poorly graded SAND, loose, brown, dry, Homogeneous. Length Recovered 1.0 ft		
10—	-3		•		2 4 4 (8)	X	D-2			Poorly graded SAND, loose, brown, dry, Homogeneous. Length Recovered 1.0 ft		
15—	-5				21 50 (50)	X	D-3 C-4			GRANITE, brown, medium grained, completely weathered, very weak rock. Length Recovered 0.8 ft GRANITE, brown, medium grained, completely weathered, very weak rock. Length Recovered 3.5 ft		
20 —	-6										-	



Start Card S-23735

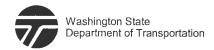
HOLE No. FV-9-04

Sheet 2 of 2

Job No. XL-2201 SR 395 Elevation 1996.2 ft (608.4 m)

Project NSLAC - Perry, Market and Fairview Structures

Depth (ft)	Meters (m)	Profile	10	Stand Penetra Blows 20	ation	40	SPT Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab Tests	Description of Material	Groundwater	Instrument
-	-7						50 (50)	Ň	D-5 C-6		GRANITE, brown, medium grained, completely weathered, very weak rock. Length Recovered 0.5 ft GRANITE, brown, medium grained, completely weathered, very weak rock. Length Recovered 1.7 ft	-	
25 —	-8						50 (50)	Ų	D-7		GRANITE, brown, medium grained, completely weathered, very weak rock. Length Recovered 0.4 ft End of test hole boring at 25.4 ft below ground elevation. This is a summary Log of Test Boring. Soil/Rock descriptions are derived from visual field identifications and laboratory test data.		
30-	-9											-	
35 —	 10		1		1							-	
	11												
40 —	— 12 -				1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1							-	
-	—13		1									-	



Start Card _____

XL-2201	SR	395	Elevation	1955.3 ft (596.0 n

HOLE No. MRK-1-00

Sheet __1__ of __3__

Project_NSLAC - Perry, Market and Fairview Structures

Driller Ben Gooding Lic#___

Site Address _

Inspector RRR

Start October 24, 2000 Completion October 24, 2000 Well ID#

____Latitude

Equipment_B-61

 Station
 LR 377+89.5
 Offset
 29.7ft Rt.
 Casing
 8-in HSA

Job No_

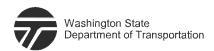
Method 8-in Hollow Stem Auger

Northing 621983

_____ Easting 2823022

_ Longitude _

Depth (ft)	Meters (m)	Profile	10	Stand Penetr Blow 20	ration	40	SPT Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab	Tests	Description of Material	Groundwater	
-	_1												3	
5	_2						3 5 5 (10)	X	D-1			Poorly graded SAND with trace silt, loose, subrounded to subangular, medium dense, gray-brown, dry to moist, homogeneous, weak HCL reaction (Qes)		
10	-3			 			4 5 8 (13)	X	D-2			Poorly graded SAND with trace of silt, subrounded to subangular, medium dense, gray-brown, dry to moist, homogeneous, weak HCL reaction (Qes)	_	
	-4			1 1 1 1 1 1 1	1	1						-		
15 —	-5								S-3			Well graded SAND, subrounded to subangular, loose, gray-brown, moist, homogeneous, weak HCL reaction (Qes)		
			1		1								4 4	



Start Card _____

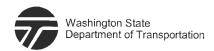
HOLE No. MRK-1-00

Sheet 2 of 3

Job No. XL-2201 SR 395 Elevation 1955.3 ft (596.0 m)

NSLAC - Perry Market and Fairview Structu

Depth (ft)	Meters (m)	Profile	1 <u>0</u>	Stand Peneti Blow	ration	40	SPT Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab Tests	Driller Ben Gooding Description of Material	Groundwater #	Instrument
							4 5 6 (11)	X	D-4	GS MC	SP, M.C.=16% Poorly graded SAND, subangular to subrounded, medium dense, gray-brown, moist, homogeneous, weak HCL reaction (Qf)		
25 — -							4 6 9 (15)	X	D-5		Well graded SAND, subrounded to subangular, medium dense, gray-brown, moist, homogeneous, weak HCL reaction (Qf)		
30 —	9 10			•			6 6 9 (15)	X	D-6	GS MC	SM, M.C. Silty SAND, subrounded to subangular, medium dense, brown to light brown, homogeneous, weak HCL reaction (Qf)		
35 —	11						8 11 16 (27)	X	D-7		Well graded SAND, subrounded to subangular, dense, gray - brown, homogeneous, strong HCL reaction (Qf)		
40 —	-12 -13						13 15 18 (33)	X	D-8		Well graded SAND, subrounded to subangular, dense, gray - brown, homogeneous, strong HCL reaction (Qf)		
				1 1									



Start Card _____

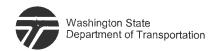
Job No_XL-2201 SR __ 395

Elevation 1955.3 ft (596.0 m)

HOLE No. MRK-1-00

Sheet __3__ of __3__

Project NSLAC - Perry, Market and Fairview Structures Driller Ben Gooding Sample No. (Tube No.) Sample Type Depth (ft) Standard Meters (m) Groundwater SPT Profile Lab Tests Penetration Blows/6" Description of Material Blows/ft (N) 20 18 Well graded SAND, subrounded to subangular, very 29 dense, gray - brown, homogeneous, strong HCL reaction 14 34 (Qf) (63)- 15 50 D-10 Well graded SAND, subrounded to subangular, very 22 25 dense, gray - brown, homogeneous, strong HCL reaction 28 (53)16 End of test hole boring at 51.5 ft below ground elevation. This is a summary Log of Test Boring. Soil/Rock descriptions are derived from visual field identifications. 55 -17 ALL XL-2201 NSLAC - PERRY, MARKET AND FARMEW STRUCTURES. GPJ SOIL. GDT 11/17/04,12:50:37 P11 - 18 60-- 19 65-- 20 -21



Start Card S 23722

Job No. XL-2201 395 SR Elevation __1949.3 ft (594.1 m)

HOLE No. MRK-2-04

Sheet __1__ of __5__

Project NSLAC - Perry, Market and Fairview Structures

Driller Fetterly

_____Lic#_2708_

Site Address VIC of Market st & E Hawthorn Rd.

Inspector Hanning

Start October 6, 2004

Completion October 7, 2004

Well ID# NA

Equipment CME 850 w/ autohammer

Station LR 376+93.9

Offset 29.5ft Rt.

Casing ____3.5"

Method Wet Rotary

Northing 621891.642

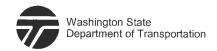
Easting 2823052.885

Latitude

Longitude

County Spokane Subsection SE/NW Range 43 EWM Section Township 26N

Depth (ft)	Meters (m)	Profile	10	Penet	ndard tration ws/ft	40	SPT Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab	Tests	Description of Material	Groundwater	Instrument
	1													
5-	-2		•				3 5 5 (10)	X	D-1			Poorly graded SAND with trace of mica, loose, olive brown, moist, Homogeneous, HCl reaction not tested Length Recovered 0.8 ft, Length Retained 0.8 ft	-	
SOIL ALL XI-2201 NSLAC - PERRY, MARKET AND FARVIEW STRUCTURES. GPJ SOIL GDT 11/17/04,12:50:41 P11 5 C C C C C C C C C C C C	_3		•				3 6 5 (11)	X	D-2	GS MC		SP, M.C. = 4% Poorly graded SAND with trace of mica, medium dense, olive brown, moist, Homogeneous, HCl reaction not tested Length Recovered 1.0 ft, Length Retained 1.0 ft	-	
AND FAIRVIEW STRUCTURES.	-4						3		D-3			Poorly graded SAND, with some wood and trace of mica.		
01 NSLAC - PERRY, MARKET ,	-5						4 5 (9)					loose, olive gray, moist, Stratified, HCl reaction not tested Length Recovered 1.0 ft, Length Retained 1.0 ft		
SOIL ALL XI-220	-6		•	1			3 4	X	D-4			Poorly graded SAND with trace of mica, loose, olive gray, moist, Stratified, HCl reaction not tested	-	



Start Card S 23722

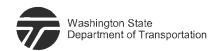
HOLE No. MRK-2-04

Sheet __2__ of __5__

Job No. XL-2201

SR 395 Elevation 1949.3 ft (594.1 m)

	Project	NSLAC	C - Perry, Market and Fairview	Structure	s			Driller Fetterly	Lic#	270
Depth (ft)	Meters (m)	Profile	Standard Penetration Blows/ft 10 20 30 40	SPT Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab Tests	Description of Material	Groundwater	
	7			5 (9)	X			Length Recovered 1.0 ft, Length Retained 1.0 ft		
25 –	-8			4 8 13 (21)	X	D-5		Poorly graded SAND with trace of mica, medium dense, olive brown, moist, Stratified, HCl reaction not tested Length Recovered 1.5 ft, Length Retained 1.5 ft		
30 —	9			13 18 21 (39)	X	D-6		Poorly graded SAND, with trace silt & fine gravel, dense, olive gray, moist, Stratified, HCl reaction not tested Length Recovered 1.5 ft, Length Retained 1.5 ft		
35 —	-10			15 20 19 (39)	X	D-7	GS MC	SP, M.C. = 14% Poorly graded SAND, dense, olive gray, moist, Stratified, HCl reaction not tested Length Recovered 1.0 ft, Length Retained 1.0 ft		
40 —	-12			15 17 21 (38)	X	D-8		Well graded SAND, dense, olive gray, moist, Stratified, HCl reaction not tested Length Recovered 1.0 ft, Length Retained 1.0 ft		
	13			10 12	X	D-9		Well graded SAND, with some medium gravel, dense, olive gray, moist, Stratified, HCl reaction not tested		



Start Card S 23722

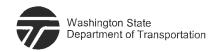
HOLE No. MRK-2-04

Sheet <u>3</u> of <u>5</u>

Job No. XL-2201 SR 39

395 Elevation 1949.3 ft (594.1 m)

Depth (ft)	Meters (m)	Profile	Per B	tandard netration Blows/ft		SPT Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab Tests	Driller Fetterly Description of Material	Groundwater #2	Instrument
	— 14 -		10 20	30	40	18 (30)	X			Length Recovered 1.0 ft, Length Retained 1.0 ft	-	
50-	—15 -					11 11 21 (32)	X	D-10		Well graded SAND, with some fine gravel, dense, olive gray, moist, Stratified, HCl reaction not tested Length Recovered 1.0 ft, Length Retained 1.0 ft		
	—16 -					13 16	•	D-11		Well graded SAND, with some medium gravel, dense,		
55 —	 17					26 (42)	Å			olive gray, moist, Stratified, HCl reaction not tested Length Recovered 1.0 ft, Length Retained 1.0 ft	-	
60 —	—18 -			•		16 20 14 (34)	X	D-12	GS MC	SW-SM, M.C. = 13% Well graded SAND with silt and gravel, dense, olive gray, moist, Stratified, HCl reaction not tested Length Recovered 1.0 ft, Length Retained 1.0 ft		
-	 19		1		1						_	
65 —	20					12 19 23 (42)	X	D-13		Silty SAND with gravel, dense, olive gray, moist, Stratified, HCl reaction not tested Length Recovered 0.8 ft, Length Retained 0.8 ft	_	
70	- 21			•		6 16	X	D-14		Poorly graded SAND, with some silt stratification and trace organics, dense, light gray, moist, Stratified, HCl	-	



Start Card S 23722

Job No. XL-2201

Elevation 1949.3 ft (594.1 m)

HOLE No. MRK-2-04

Sheet __4 of __5_

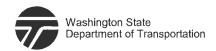
Project NSLAC - Perry, Market and Fairview Structures Driller Fetterly Lic# 2708 Standard Sample Type Depth (ft) Sample No. Meters (m) (Tube No.) SPT Profile Penetration Blows/6" Description of Material Blows/ft (N) 20 30 40 15 reaction not tested, Note bottom 0.4ft sandy silt, pinkish (31)Length Recovered 1.2 ft, Length Retained 1.2 ft 22 Clayey SAND with gravel, dense, light greenish gray, D-15 16 moist, Stratified, HCI reaction not tested 75 23 Length Recovered 1.5 ft, Length Retained 1.5 ft (39)23 24 D-16 10 GS SC, M.C. = 22%, PI = 10 Clayey SAND, dense, light greenish gray, moist, 18 MC 80 26 AL Homogeneous, HCI reaction not tested (44)Length Recovered 1.5 ft, Length Retained 1.5 ft SOIL ALL XI-2201 NSLAC - PERRY, MARKET AND FAIRVIEW STRUCTURES GPJ SOIL GDT 11/17/04,12:50:41 P11 25 12 D-17 Clayey SAND, and fine gravel, dense, light greenish gray, 20 moist, Homogeneous, HCl reaction not tested, 85-25 Completely weathered granite. 26 Length Recovered 1.5 ft, Length Retained 1.5 ft 27 11 D-18 Clayey SAND, with some fine gravel, dense, greenish 20 gray, moist, Homogeneous, HCl reaction not tested 26 Length Recovered 1.0 ft, Length Retained 1.0 ft (46)28

Clayey SAND, dense, greenish gray, moist,

Homogeneous, HCI reaction not tested

12

22



Start Card S 23722

HOLE No. MRK-2-04

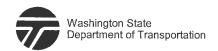
Sheet __5__ of __5__

Job No. XL-2201

395___ SR

Elevation 1949.3 ft (594.1 m)

	Project	NSLAC	- Perry,	Market	and Fairv	/iew Struc		T			_	Driller Fetterly L	ic#_2	2708
Depth (ft)	Meters (m)	Profile	10	Standa Penetra Blows 20	ation	SF Blow (N	T s/6"	Sample Type	Sample No. (Tube No.)	Lab	Tests	Description of Material	Groundwater	hostrument
	- 29		1	1 1 1 1 1 1 1 1	1 1/	(4)	5 7)	X				Length Recovered 1.0 ft, Length Retained 1.0 ft		
100	- 30			1 1 1 1 1 1 1	•	1. 1: 2: (3:	2 3	X	D-20			Clayey SAND, and fine gravel, dense, greenish gray, moist, Homogeneous, HCl reaction not tested Length Recovered 1.5 ft, Length Retained 1.5 ft	-	
	-31		I I	1								End of test hole boring at 100.5 ft below ground elevation.	- -	
105—	32											This is a summary Log of Test Boring. Soil/Rock descriptions are derived from visual field identifications and laboratory test data.	-	
110 —	-33													
	-34		1											
115—	-35		1											
_	- 36			1									-	



Job No_XL-2201 SR

Site Address Vic. of Market st & East Hawthorne

LOG OF TEST BORING

Elevation 1954.5 ft (595.7 m)

Start Card S 23722

HOLE No. MRK-3-04

Inspector Hanning

Sheet __1__ of __5__

Driller Harvey Lic# 2599

Project NSLAC - Perry, Market and Fairview Structures

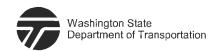
Start September 28, 2004 Completion September 29, 2004 Well ID# NA Equipment CME 850 w/ autohammer

Station LR 378+13.1 Offset 1.2ft Rt. Casing 3.5"

395___

Method Wet Rotary

		Profile	10	Penetration Blows/ft 20 30	40	SPT Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab	Tests	Description of Material	Groundwater	
	-1											-	
5	-2		•			2 3 4 (7)	X	D-1			Poorly graded SAND with silt and trace of mica, loose, olive brown, moist, Homogeneous, HCl reaction not tested Length Recovered 1.0 ft, Length Retained 1.0 ft		
10	-3		*			2 3 3 (6)	X	D-2			Poorly graded SAND with silt and trace of mica, loose, olive brown, moist, Stratified, HCl reaction not tested Length Recovered 0.7 ft, Length Retained 0.7 ft		
15—	-4		+			2 3 3 (6)	X	D-3			Poorly graded SAND with silt and trace of mica, loose, olive gray, moist, Stratified, HCl reaction not tested Length Recovered 0.8 ft, Length Retained 0.8 ft		
	-5					3 3	X				olive gray, moist, Stratified, HCl reaction not tested		



Start Card S 23722

HOLE No. MRK-3-04

Sheet 2 of 5

Job No_XL-2201 SR 395 Elevation 1954.5 ft (595.7 m)

Project NSLAC - Perry, Market and Fairview Structures

	Project	NSLAC	- Perry, Market and F	airview Structure	es	U		Driller _ Harvey	Lic#_2	599_
Depth (ff)	Meters (m)	Profile	Standard Penetration Blows/ft 10, 20 30	SPT Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab Tests	Description of Material	Groundwater	Instrument
8	7			2 3 9 (12)	X	D-4	GS MC	SP-SM, M.C. = 15% Poorly graded SAND with silt and trace of mica, medium dense, olive gray, moist, Stratified, HCl reaction not tested Length Recovered 0.6 ft, Length Retained 0.6 ft	-	
25 -	8			2 5 7 (12)	X	D-5		Poorly graded SAND with silt and trace of mica, medium dense, olive gray, moist, Stratified, HCl reaction not tested Length Recovered 1.0 ft, Length Retained 1.0 ft		
30 -	9 10			5 7 9 (16)	X	D-6		Poorly graded SAND with silt and trace of mica, medium dense, olive gray, moist, Stratified, HCl reaction not tested Length Recovered 1.0 ft, Length Retained 1.0 ft		
35 -	= 11			7 15 21 (36)	X	D-7		Well graded SAND with silt, dense, olive gray, moist, Stratified, HCl reaction not tested Length Recovered 1.3 ft, Length Retained 1.3 ft		
35 -	-12 -13			17 18 23 (41)	X	D-8		Well graded SAND with silt and some fine gravel & silt, dense, olive gray, moist, Stratified, HCl reaction not tested Length Recovered 0.8 ft, Length Retained 0.8 ft		
45-	-			i.					-	



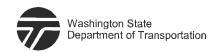
Start Card S 23722

HOLE No. MRK-3-04

Sheet <u>3</u> of <u>5</u>

Job No. XL-2201 SR 395 Elevation 1954.5 ft (595.7 m)

	Project	NSLAC	- Perry, Marke	t and Fairview	Structure	S			Driller Harvey	Lic#_2	2599
Depth (ft)	Meters (m)	Profile	Stand Penetr Blow 10 20	ation	SPT Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab Tests	Description of Material	Groundwater	Instrument
	14				17 17 18 (35)	X	D-9		Well graded SAND with silt and some medium gravel & silt, dense, olive gray, moist, Stratified, HCl reaction not tested Length Recovered 1.0 ft, Length Retained 1.0 ft		
50-	-15		•		14 12 13 (25)	X	D-10		Well graded SAND with silt and some fine gravel, dense, olive gray, moist, Stratified, HCl reaction not tested Length Recovered 1.0 ft, Length Retained 1.0 ft		
5 A2	-16				7 18 21 (39)	X	D-11		Well graded SAND with silt and some fine gravel, dense, olive gray, moist, Stratified, HCl reaction not tested Length Recovered 1.3 ft, Length Retained 1.3 ft		
RUCTURES.GPJ SOIL, GDT 2/10/05,10:53:46 A2	-18 -19				17 17 29 (46)	X	D-12		Well graded SAND with silt and some fine gravel, dense, olive gray, moist, Stratified, HCl reaction not tested Length Recovered 1.0 ft, Length Retained 1.0 ft		
SOIL ALL XL-2201 NSLAC - PERRY, MARKET AND FAIRVIEW STRUCTURES, GPJ 9 9	_				16 20 25 (45)	X	D-13	GS MC	SW-SM, M.C. = 13% Well graded SAND with silt and some fine gravel, dense, olive gray, moist, Stratified, HCl reaction not tested Length Recovered 1.3 ft, Length Retained 1.3 ft		
SOIL ALL XL-2201 NSLA	21									-	



Start Card S 23722

HOLE No. MRK-3-04

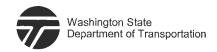
Sheet <u>4</u> of <u>5</u>

Job No_XL-2201_____

SR 395

Elevation 1954.5 ft (595.7 m)

-22 -23 -24	16 18 (34)	D-15	Well graded SAND with silt and some fine gravel, dense, olive gray, moist, Stratified, HCl reaction not tested Length Recovered 1.0 ft, Length Retained 1.0 ft Well graded SAND with silt and some gravel, dense, olive gray, moist, Stratified, HCl reaction not tested	
-23	23 21		Well graded SAND with silt and some gravel, dense, olive gray, moist. Stratified, HCl reaction not tested	
-24	/		Length Recovered 0.8 ft, Length Retained 0.8 ft	-
-25	24 17 15 (32)	1 1	Well graded SAND with silt and gravel, dense, olive gray, moist, Stratified, HCl reaction not tested Length Recovered 1.0 ft, Length Retained 1.0 ft	
85——26	11 14 16 (30)		Well graded SAND with silt and some gravel, dense, olive gray, moist, Stratified, HCl reaction not tested Length Recovered 1.5 ft, Length Retained 1.5 ft	
90 - 28	11 18 23 (41)	v	GRANITE, pale brown, medium grained, completely weathered, very weak rock. Length Recovered 1.5 ft, Length Retained 1.5 ft	



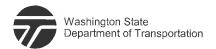
Start Card S 23722

HOLE No. MRK-3-04

Sheet __5__ of __5_

Job No. XL-2201 SR 395 Elevation 1954.5 ft (595.7 m)

	Project	NSLAC	- Perry,	Market	t and I	Fairview	Structure	s				Driller <u>Harvey</u> L	_ic#	2599
Depth (ft)	Meters (m)	Profile	10	Stand Penetr Blow 20	ration	40	SPT Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab	Tests	Description of Material	Groundwater	Instrument
	- 29		1 1 1 1 1 1		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		12 18 28 (46)	X	D-19			GRANITE, pale brown, medium grained, completely weathered, very weak rock. Length Recovered 1.5 ft, Length Retained 1.5 ft	-	
100 —	-30	/ / / / / / / / / / / / / / / / / / /	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	>>4	14 25 32 (57)	X	D-20			GRANITE, pale brown, medium grained, completely weathered, very weak rock. Length Recovered 1.5 ft, Length Retained 1.5 ft		
: :-	-31		 									End of test hole boring at 101.5 ft below ground elevation. This is a summary Log of Test Boring. Soil/Rock descriptions are derived from visual field identifications and laboratory test data. Bailed hole, no recharge.		
105 —	-32		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1										
110 —	-33		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1											
	-34		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1									-	
115 —	-35												-	
-	- 36		1	1										



Elevation 1958.1 ft (596.8 m)

Start Card S 23722

HOLE No. MRK-4-04

Sheet __1__ of __5_

Project NSLAC - Perry, Market and Fairview Structures

SR 395___

Driller Harvey Lic# 2599

Site Address Vic. of Market st. & E. Hawthorn

Inspector Hanning

Longitude

Start September 29, 2004 Completion September 30, 2004 Well ID# NA

Equipment CME 850 w/ autohammer

Station LR 378+60.1

Job No. XL-2201

Offset 70.4ft Rt. Casing 3.5"

Method Wet Rotary

Northing 622064.337

Easting 2823035.02

Latitude

Depth (ft)	Meters (m)	Profile	10	Standa Penetral Blows/ 20	tion	SPT Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab Tests	Description of Material	Groundwater	- de de la constant
5	1 2		*			2 4 4 (8)	X	D-1		Poorly graded SAND with silt and trace of mica, loose, olive brown, moist, Homogeneous, HCl reaction not tested Length Recovered 0.8 ft, Length Retained 0.8 ft		
10 —	-3		•			2 2 3 (5)	X	D-2		Poorly graded SAND with silt and trace of mica, loose, olive gray, moist, Stratified, HCl reaction not tested Length Recovered 0.8 ft, Length Retained 0.8 ft		
15—	—4 —5		•			2 2 4 (6)	X	D-3		Poorly graded SAND with silt and trace of mica, loose, olive gray, moist, Stratified, HCl reaction not tested Length Recovered 0.5 ft, Length Retained 0.5 ft		
-	- 6		•			3 4	X	D-4		Poorly graded SAND with silt and trace of mica, loose, olive gray, moist, Stratified, HCl reaction not tested		



Start Card S 23722

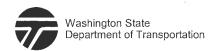
HOLE No. MRK-4-04

Sheet 2 of 5

Job No. XL-2201 SR 395 Elevation 1958.1 ft (596.8 m)

NSLAC - Perry Market and Fairview Structur

Depth (ft)	Meters (m)	Profile	10	Stand Penetr Blow	ration	40	SPT Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab Tes ts	Description of Material	Groundwater	
	-7						4 (8)	X			Length Recovered 1.0 ft, Length Retained 1.0 ft	-	
25	8						4 5 6 (11)	X	D-5		Poorly graded SAND with silt and trace of mica, medium dense, olive gray, moist, Stratified, HCl reaction not tested Length Recovered 1.0 ft, Length Retained 1.0 ft		
30 —	- 9						4 5 6 (11)	X	D-6		Poorly graded SAND with silt and trace of mica, medium dense, olive gray, moist, Stratified, HCl reaction not tested Length Recovered 0.7 ft, Length Retained 0.7 ft		
Ī	<u> </u>			1		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1							
35 —	-11						4 5 8 (13)	X	D-7	GS MC	SW-SM, M.C. = 14% Well graded SAND with silt and trace of mica, medium dense, olive gray, moist, Stratified, HCl reaction not tested Length Recovered 0.8 ft, Length Retained 0.8 ft		
40 —	-12						17 15 21 (36)	X	D-8	GS MC	SW-SM, M.C. = 9% Well graded SAND with silt, dense, olive gray, moist, Stratified, HCl reaction not tested Length Recovered 1.0 ft, Length Retained 1.0 ft		
-	— 13		1	1 1 1			V					-	
	5		l I	1	1	>>	16 33	Y	D-9		Well graded SAND with silt, very dense, olive gravel, moist, Stratified, HCl reaction not tested	-	



Start Card S 23722

HOLE No. MRK-4-04

Sheet <u>3</u> of <u>5</u>

Job No. XL-2201

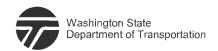
SR <u>395</u>

Elevation 1958.1 ft (596.8 m)

Project NSLAC - Perry, Market and Fairview Structures

Driller Harvey Lin# 2599

£)	(m)	9		Stan Penet			SPT	Туре	o No No.)	0	ts		water	
Depth (ft)	Meters (m)	Profile	10	Blov 20		40	Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab	Tests	Description of Material	Groundwater	
	— 14		1	1			20 (53)	X				Length Recovered 1.0 ft, Length Retained 1.0 ft		
50 —	- 15						16 17 25 (42)	X	D-10			Well graded SAND with silt and some gravel, dense, olive gray, moist, Stratified, HCl reaction not tested Length Recovered 1.0 ft, Length Retained 1.0 ft		
	-16		1		•		16		D-11			Well graded SAND with silt and gravel, dense, olive gray,		
55—	17						15 13 (28)		2			moist, Stratified, HCl reaction not tested Length Recovered 1.0 ft, Length Retained 1.0 ft		
60 —	-18					•	8 17 26 (43)	X	D-12			Well graded SAND with silt, dense, olive gray, moist, Stratified, HCl reaction not tested Length Recovered 0.8 ft, Length Retained 0.8 ft		
	19		 	1 1			30		D-13			Well graded SAND, with silt and gravel, very dense, olive	-	
65 —	20			1 1 1 1 1 1			32 23 (55)	X	0			gray, moist, Stratified, HCI reaction not tested Length Recovered 1.2 ft, Length Retained 1.2 ft		
	- 21			1 1		+	16 20	Y	D-14			Well graded SAND with silt, dense, olive gray, moist, Stratified, HCl reaction not tested		



Start Card S 23722

HOLE No. MRK-4-04

Sheet <u>4</u> of <u>5</u>

Job No. XL-2201 SR 395 Elevation 1958.1 ft (596.8 m)

Depth (ft)	Meters (m)	Profile	10	Standa Penetra Blows	ation	40	SPT Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab Tests	Driller Harvey Description of Material	Groundwater #2	
	22			1		1	24 (44)	X			Length Recovered 1.2 ft, Length Retained 1.2 ft		
75—	23						18 24 24 (48)	X	D-15		Well graded SAND, with silt, dense, olive gray, moist, Stratified, HCl reaction not tested Length Recovered 1.2 ft, Length Retained 1.2 ft		
80 —	— 24 -						18 24 20 (44)	X	D-16	GS MC	SW-SM, M.C. = 10% Well graded SAND with silt and gravel, dense, olive gray, moist, Stratified, HCl reaction not tested Length Recovered 0.6 ft, Length Retained 0.6 ft	-	
85 —	— 25 - — 26						18 25 25 (50)	X	D-17		Silty SAND, with silt and gravel, very dense, olive gray, moist, Stratified, HCl reaction not tested Length Recovered 1.0 ft, Length Retained 1.0 ft		
90—	— 27						11 18 26 (44)	X	D-18		Poorly graded SAND with silt, dense, olive gray, moist, Homogeneous, HCl reaction not tested Length Recovered 0.5 ft, Length Retained 0.5 ft		
	— 28 -		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1			>>-	11		D-19		Well graded SAND with silt, stratified with silty sand, very		



120

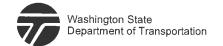
LOG OF TEST BORING

Start Card S 23722

Job No. XL-2201 395 Elevation _1958.1 ft (596.8 m)

HOLE No. MRK-4-04

Sheet __5__ of __5_ Project_NSLAC - Perry, Market and Fairview Structures Driller Harvey Lic#_2599 Sample Type Groundwater Meters (m) Standard Sample No. Depth (ft) (Tube No.) SPT Instrument Tests Penetration Lab Blows/6" Description of Material Blows/ft (N) 29 not tested (59)Length Recovered 1.0 ft, Length Retained 1.0 ft 30 D-20 50/4" No Recovery (50)100 -End of test hole boring at 99.4 ft below ground elevation. -31 This is a summary Log of Test Boring. Soil/Rock descriptions are derived from visual field identifications and laboratory test data. 105 32 SOIL ALL XL-2201 NSLAC - PERRY, MARKET AND FAIRVIEW STRUCTURES.GPJ SOIL.GDT 12/8/04,12:54;10 P12 -33 110--34 35 115-- 36



Elevation 1932.4 ft (589.0 m)

Start Card S 23985

HOLE No. PRK-7-04

Sheet __1__ of __5__

Driller Kerry Cooper Lic# 2552

Project NSLAC - Parksmith Structures

Site Address Vicinity of Park Smith and Market Street

Inspector Cleo Andrews

Start July 19, 2004

Completion July 19, 2004

395

____Well ID# ____

Equipment CME 45 w/ autohammer

Station LL 393+28.97

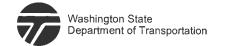
Job No. XL-2201

Offset 54.74' Lt. Casing HQ 3" ID x 105.0'

Method Wet Rotary

County Spokane Subsection NE 1/4 of the NW 1/4 Section 15 Range 43 EWM Township 26

Depth (ft)	Meters (m)	Profile	10	Standard Penetratio Blows/ft 20 30	n	SPT Blows/6" (N)	Sample Type	Sample No.	Lab	Tests	Description of Material	Groundwater	Instrument
5-	-1		•			2 2 3 (5)	X	D-1			Well graded SAND with silt, loose, brown, moist, Stratified,0.0' to 4.0' Poorly graded Sand with some gravel as indicated by drilling and wash return. 100% drilling fluid return. Length Recovered 1.0 ft, Length Retained 1.0 ft.		
2/1/05,1:26,48 P2 - 01	3				•	25 30 16 (46)	X	D-2	GS MC		SM, M.C. = 11% Silty SAND, dense, brown, moist, Stratified, HCl reaction not tested. Very little drilling fluid loss. Length Recovered 1.5 ft, Length Retained 1.0 ft.		
SOIL XL-2201 NSLAC - PARKSMITH STRUCTURES.GPJ SOIL.GDT 2/11/05,1:28:48 P2 10 10 10	-4			*		8 11 12 (23)	X	D-3	GS MC		SW-SM, M.C. = 16% Well graded SAND with silt, with silty Sand lens, medium dense, brown, moist, Laminated. Length Recovered 1.5 ft, Length Retained 1.0 ft.	- - - -	
SOIL XL-2201 NSLAC	- -6		1	•	1	11 11	X	D-4			Silty SAND, medium dense, brown, moist, Homogeneous.		



Start Card S 23985

HOLE No. PRK-7-04

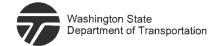
Job No. XL-2201

SR 395

Elevation _1932.4 ft (589.0 m)

Sheet __2__ of __5_

Project	NSLAC	C - Parksmith Structures					Driller Kerry Cooper	Lic#_2	2552
Depth (ft) Meters (m)	Profile	Standard Penetration Blows/ft 10 20 30 40	SPT Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab Tests	Description of Material	Groundwater	Instrument
-			8 (19)	X			Length Recovered 1.5 ft, Length Retained 1.0 ft,		
25 —			12 18 16 (34)	X	D-5	GS MC	SM, M.C. = 26% Silty SAND, dense, brown, moist, Homogeneous. Length Recovered 1.5 ft, Length Retained 1.0 ft.		
30 —			13 13 16 (29)	X	D-6		Poorly graded SAND with silt, dense, brown, moist, Stratified. Length Recovered 1.5 ft, Length Retained 1.0 ft.		
35 — 11			13 13 16 (29)	X	D-7		Poorly graded SAND with silt, medium dense, brown, moist, Stratified with 0.5 of silty sand. Length Recovered 1.5 ft, Length Retained 1.0 ft.		
40-12			13 13 10 (23)	X	D-8	GS MC	SP-SM, M.C. = 22% Poorly graded SAND with silt, medium dense, brown, moist, Homogeneous. Length Recovered 1.5 ft, Length Retained 1.0 ft.		
13 			11 33	X	D-9		Well graded GRAVEL with sand, with sandy Silt laminated with fine grained sand, subrounded, very	_[



Start Card S 23985

HOLE No. PRK-7-04

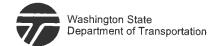
Job No. XL-2201

395

Elevation 1932.4 ft (589.0 m)

Sheet 3 of 5

Project NSLAC - Parksmith Structures Driller Kerry Cooper Lic# 2552 Sample Type Standard Sample No. Depth (ft) Groundwater Meters (m) SPT (Tube No.) Instrument Profile Tests Penetration Lab Blows/6" Description of Material Blows/ft (N) 50/5 dense, brown, moist. (Took moister can sample same 0. (50)depth from sandy silt zone, retained 3"). Length Recovered 1.4 ft, Length Retained 1.3 ft. 15 37 D-10 Well graded SAND with silt and gravel, subrounded, very 50/4 dense, grayish brown, moist, Homogeneous. 50 (50)(Approximately 1 to 3% drilling fluid loss at times). Length Recovered 0.5 ft, Length Retained 0.5 ft. 16 75/6 D-11 Well graded SAND with silt and gravel, subrounded, very (75)dense, grayish brown, moist, Homogeneous. 55 Length Recovered 0.5 ft, Length Retained 0.5 ft. 18 33 D-12 Well graded SAND with silt and gravel, subrounded, very 25 dense, grayish brown, moist, Homogeneous. Trace of SOIL XL-2201 NSLAC - PARKSMITH STRUCTURES.GPJ SOIL.GDT 2/11/05,1:26:48 P2 60 32 brownish orange stain. (57)Length Recovered 1.2 ft, Length Retained 1.0 ft. 19 16 D-13 Well graded SAND with silt and gravel, very dense, 31 grayish brown, moist, Homogeneous. 65 25 Length Recovered 1.0 ft, Length Retained 1.0 ft. (56)20 35 D-14 Well graded SAND with silt and gravel, subrounded, very 25 dense, grayish brown, moist, Homogeneous.



Job No. XL-2201

LOG OF TEST BORING

Start Card S 23985

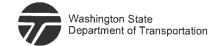
HOLE No. PRK-7-04

395

Elevation _1932.4 ft (589.0 m)

Sheet <u>4</u> of <u>5</u> Project_NSLAC - Parksmith Structures Driller _Kerry Cooper

Depth (ft)	Meters (m)	Profile	Standar Penetrat Blows/1	on	SPT Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab Tests	Description of Material	Groundwater	
_	22				26 (51)	X			Length Recovered 1.0 ft, Length Retained 1.0 ft.	-	
75 —	-23			>>4	27 26 28 (54)	X	D-15		Well graded SAND with silt, very dense, grayish brown, moist, Homogeneous. Length Recovered 1.2 ft, Length Retained 1.0 ft.		
80 —	24 25			>>	30 22 33 (55)		D-16		Well graded SAND with silt and gravel, very dense, brown, moist, Homogeneous, no HCl reaction. 0.1' layer of sandy silt. Length Recovered 1.5 ft, Length Retained 1.0 ft.		
85 —	- 26			>>•	26 27 36 (63)		D-17		Well graded SAND with silt and gravel, very dense, brown, moist, Homogeneous. (Encountered some coarser gravel at 87.0' as indicated by drilling). Length Recovered 1.0 ft, Length Retained 1.0 ft.		
90 —	-27 -28				33 50/4 (50)	X	D-18		Well graded SAND with silt and gravel, very dense, brown, moist, Homogeneous. Length Recovered 0.8 ft, Length Retained 0.8 ft.		
-									GRANITE boulder.		



Start Card S 23985

HOLE No. PRK-7-04

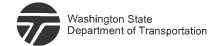
Sheet <u>5</u> of <u>5</u>

Job No. XL-2201

SR 395

Elevation 1932.4 ft (589.0 m)

Depth (ft)	Meters (m)	Profile	10	Stand Peneti Blow	ration	40	SPT Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab	Description of Material	Groundwater	Inctaiment
	- 29 - - 30						24 30 24 (54)		D-19		Well graded SAND with silt and gravel, very dense, grayish brown, moist, Homogeneous. Length Recovered 1.0 ft, Length Retained 1.0 ft.	-	
100 —	-				-	>>	55 51/6 (51)	X	D-20		Well graded SAND with silt and gravel, very dense, grayish brown, moist, Homogeneous. Trace of silt. Length Recovered 1.0 ft, Length Retained 1.0 ft		
-	-31		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		1 1 1 1 1 1 1 1						End of test hole boring at 100 ft below ground elevation. This is a summary Log of Test Boring. Soil/Rock descriptions are derived from visual field identifications and laboratory test data.	<u>-</u>	
05	- 32		i 								Water in casing at 15.0', pulled 5.0' of casing. Bailed hole to 98.0', waited 15 minutes dry hole. Ended and abandoned test boring at 100.0' below ground elevation. 7/19/04.	-	
	- 33										,	-	
10			1	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1								-	
	- 34		1									-	
15—	-35		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1										
-			i 1 1	i i	į							-	



Job No_XL-2201

LOG OF TEST BORING

Elevation <u>1921.0 ft (585</u>.5 m)

Start Card S-23985

HOLE No. PRK-8-04

Sheet __1__ of __5__

Driller Joe Judd Lic# 2454

Inspector Dave Nelson

Project NSLAC - Parksmith Structures

Start July 19, 2004 Completion July 20, 2004

__395

Well ID# ______ Equipment CME 850 w/ auto hammer

Station LR 394+47.74

Site Address Parksmith & Market

Offset 35.53' Rt. Casing 6" x 50.0 & 4" x 99.0

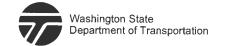
Method Wet Rotary

Longitude_

Northing 623431.63 Easting 2822208.81

Latitude _____

Depth (ft)	Meters (m)	Profile	Standard Penetration Blows/ft 10 20 30 40	SPT Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab Tests	Description of Material	Groundwater	
5	—1 —2			3 6 6 (12)	X	D-1	GS MC	SP-SM, M.C. = 15% Poorly graded SAND with silt, medium dense, brown, dry, Homogeneous. Length Recovered 1.0 ft.		
10	-3			6 7 8 (15)	V	D-2		Well graded SAND with silt, medium dense, brown, dry, Homogeneous. Length Recovered 1.0 ft.		
15—	-4 -5			2 3 12 (15)		D-3	GS MC	ML, M.C. = 32% Sandy SILT, medium dense, brown, wet, Homogeneous. Length Recovered 1.0 ft.		
-	- 6			12 13	Y	D-4		Well graded SAND with silt, dense, brown, dry, Homogeneous.	-	



Start Card S-23985

HOLE No. PRK-8-04

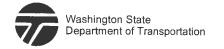
Sheet <u>2</u> of <u>5</u>

Job No. XL-2201

395 SR

Elevation 1921.0 ft (585.5 m)

	Project	NOLAC -	Parksmith Structures	T			I	Driller Joe Judd	Lic#_2	245
Depth (ft)	Meters (m)	Profile	Standard Penetration Blows/ft 10 20 30 40	SPT Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab	Description of Material	Groundwater	1 13
				14 (27)	X			Length Recovered 1.0 ft,	-	in the state of th
25—				9 13 16 (29)	¥ A	D-5	GS MC	SW-SM, M.C. = 14% Well graded SAND with silt, dense, brown, dry, Homogeneous. Length Recovered 1.0 ft.		
30 —	-9			8 13 15 (28)	X	D-6	GS MC	SM , M.C. = 20% Silty SAND, dense, brown, dry, Homogeneous. Length Recovered 1.0 ft.		
35 —	-10 -			5 12 25 (37)	V	D-7		Well graded SAND with silt and gravel, dense, brown, moist, Stratified. Silt stratified Length Recovered 1.0 ft		
40-	12			16 17 22 (39)	X	D-8		Well graded SAND with silt and gravel, dense, brown, moist, Homogeneous. Boulder from 36.0' to 37.0'. Length Recovered 0.5 ft.		
45	-13			12 15	Y	D-9		Well graded SAND with silt, dense, brown, moist, Homogeneous.	_	



XL-2201 NSLAC - PARKSMITH STRUCTURES GPJ SOIL GDT 2/11/05,1:28:50 P2

LOG OF TEST BORING

Start Card S-23985

HOLE No. PRK-8-04

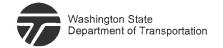
Sheet __3__ of __5__

Job No. XL-2201 395 SR

Elevation 1921.0 ft (585.5 m)

Project NSLAC - Parksmith Structures

Driller Joe Judd Lic#_2454 Sample Type Standard Depth (ft) Meters (m) Sample No. (Tube No.) Groundwater SPT Profile Instrument Tests Penetration Lab Blows/6" Description of Material Blows/ft (N) 30 29 Length Recovered 0.8 ft. (44)-15 D-10 10 GS SW-SM, M.C. = 12% 15 MC Well graded SAND with silt, dense, brown, moist, 50 19 Homogeneous. (34)Length Recovered 1.0 ft. 16 12 D-11 Well graded SAND with silt, dense, brown, moist, 16 Homogeneous. 55 20 Length Recovered 1.0 ft. (36)18 Well graded SAND with silt, dense, brown, moist, 13 D-12 16 Homogeneous. 60 16 Length Recovered 1.0 ft. (32)19 13 D-13 Well graded SAND with silt, dense, brown, moist, 20 Homogeneous. 65 25 Length Recovered 1.0 ft. (45)-20 Well graded SAND with silt, dense, brown, moist, 17 D-14 22 Homogeneous.



Start Card _S-23985

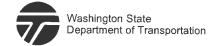
HOLE No. PRK-8-04

Sheet __4__ of __5_

Job No. XL-2201 SR 395

Elevation _ 1921.0 ft (585.5 m)

	Project	NSLAC	- Parks	mith Str	uctures	5	T					Driller <u>Joe Judd</u>	Lic#_24	54
Depth (ft)	Meters (m)	Profile	10	Stand Penetra Blows	ation s/ft	40	SPT Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab	Tests	Description of Material	Groundwater	Instrument
-	- 22		1	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	I I I I I		(46)	X				Length Recovered 1.0 ft.	-	
75 —	-23					>>4	21 27 32 (59)	X	D-15			Well graded SAND with silt, very dense, brown, moist, Homogeneous. Length Recovered 1.0 ft.		
30 -	 24					>>4	20 25 27 (52)	X	D-16			Well graded SAND with silt, very dense, brown, moist, Homogeneous. Length Recovered 1.0 ft.		
85-	-25 -26				•		13 15 17 (32)	X	D-17	G M		SP-SM, M.C. = 15% Poorly graded SAND with silt, dense, brown, moist, Homogeneous. Length Recovered 1.0 ft.		
90 -	- 27						14 16 16 (32)	X	D-18			Well graded SAND with silt, dense, brown, moist, Homogeneous. Length Recovered 1.0 ft.		
	-28						15 16	Y	D-19			Well graded SAND with silt, dense, brown, moist, Homogeneous.		



Start Card S-23985

HOLE No. PRK-8-04

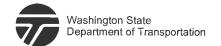
Job No. XL-2201

395 SR

Elevation _1921.0 ft (585.5 m)

Sheet ___5__ of ___5__

		NOLAC	- Parksmith Structur	es		0		T		Driller <u>Joe Judd</u>	_ic#2	245
Depth (ft)	Meters (m)	Profile	Standard Penetration Blows/ft 10 20 30	40	SPT Blows/6" (N)	Sample Type	Sample No.	Lab	Tests	Description of Material	Groundwater	
100-	- 30			•	23 (39) 15 17 30 (47)	X	D-20			Well graded SAND with silt, dense, brown, moist, Homogeneous. Length Recovered 1.0 ft.		
-	−31									End of test hole boring at 100.5 ft below ground elevation. This is a summary Log of Test Boring. Soil/Rock descriptions are derived from visual field identifications and laboratory test data. Bailed hole to 96.0 with no recharge.		
105	— 32 -									balled fible to 90.0 will fib feeliarge.		
110	-33											
- - - -	−34				**						-	
115	35											
	- 36											



Job No_XL-2201_

LOG OF TEST BORING

Elevation 1924.8 ft (586.7 m)

Start Card _S-23985

HOLE No. PRK-9-04

Sheet __1__ of __5_

Driller Joe Judd Lic# 2454

Project NSLAC - Parksmith Structures

Site Address Parksmith & Market

Inspector Dave Nelson

Start July 21, 2004 Completion July 21, 2004

395

Well ID# _____ Equipment CME 850 w/ autohammer

Station LR 395+53.87

Offset 52.15' Rt. Casing 4" x 99.0

Method Wet Rotary

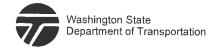
Northing 623530.4 Easting 2822166.67

SR

Latitude

_ Longitude _

Depth (ft)	Meters (m)	Profile	10	Stand Penetr Blow 20	ration	40	SPT Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab Tests	Description of Material	Groundwater	Instrument
5—	-1 -2		*				4 3 3 (6)	X	D-1		Poorly graded SAND with silt and gravel , loose, brown, dry, Homogeneous. Length Recovered 1.0 ft.		
10-	-3						3 6 10 (16)	X	D-2	GS MC	SP-SM, M.C. = 20% Poorly graded SAND with silt, medium dense, brown, dry, Homogeneous. Length Recovered 1.0 ft.		
15	-4 -5						7 9 12 (21)	X	D-3	GS MC	SM, M.C. = 15% Silty SAND, medium dense, brown, dry, Homogeneous. Length Recovered 1.0 ft.		
-	-6		1		\		8 16	•	D-4	GS MC	SW-SM, M.C. = 20% Well graded SAND with silt, dense, brown, moist,	-	



Start Card S-23985

HOLE No. PRK-9-04

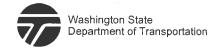
Job No_XL-2201

395

Elevation 1924.8 ft (586.7 m)

Sheet 2 of 5

Project NSLAC - Parksmith Structures Driller Joe Judd Lic#_2454 Sample Type Standard Meters (m) Sample No. Groundwater Depth (ft) (Tube No.) SPT Instrument Profile Tests Penetration Lab Blows/6" Description of Material Blows/ft (N) 20 30 10 40 19 Homogeneous. Length Recovered 1.0 ft. (35)Silty SAND, medium dense, brown, moist, 12 D-5 17 Homogeneous. 25 15 Length Recovered 1.0 ft. (32)8 7 D-6 Silty SAND, dense, brown, moist, Homogeneous. 9 12 Length Recovered 1.0 ft. 30 13 (25)10 D-7 GS SM, M.C. = 21% 14 МС Silty SAND, dense, brown, moist, Homogeneous. SOIL XL-2201 NSLAC - PARKSMITH STRUCTURES.GPJ SOIL,GDT 2/11/05,1:26:51 P2 35 13 Length Recovered 1.0 ft. (27)D-8 Silty SAND, medium dense, brown, moist, -12 13 Homogeneous. 40 11 Length Recovered 1.0 ft. (24)13 D-9 Well graded SAND with silt and gravel, dense, grey, 14 moist, Homogeneous.



Start Card S-23985

HOLE No. PRK-9-04

Job No. XL-2201 SR 395

Elevation <u>1924.8 ft (586.7 m)</u>

Sheet <u>3</u> of <u>5</u>

i	Projec	NSLAC	C - Parksm	ith Stru	ıctures					Sheet 3 of 5 Driller Joe Judd	Lic# _ 2	454
Depth (ft)	Meters (m)	Profile	10	Standa Penetrat Blows/	tion	SPT Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab Tests	Description of Material	Groundwater	Instrument
72	14					14 (28)	X			Length Recovered 1.0 ft.		
50 —	— 15 —					13 16 21 (37)	X	D-10 *		Well graded SAND with silt and gravel, dense, grey, moist, Homogeneous. Length Recovered 1.0 ft.		
-	—16 —					13 17	7	D-11		Well graded SAND with silt and gravel, dense, grey, moist, Homogeneous.	-	
55 —	— 17 —					20 (37)				Length Recovered 1.0 ft.		
60 —	— 18				•	14 18 23 (41)	X	D-12		Well graded SAND with silt, dense, grey, moist, Homogeneous. Length Recovered 1.0 ft.	-	
-	— 19		1								-	
65 —	-20					15 16 18 (34)	X	D-13		Well graded SAND with silt, dense, grey, moist, Homogeneous. Length Recovered 1.0 ft.	-	
70	21		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1 1 1 1	•	12 18	X	D-14		Well graded SAND with silt, dense, grey, moist, Homogeneous.		



Start Card S-23985

HOLE No. PRK-9-04

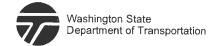
Job No. XL-2201

395 SR

Elevation __1924.8 ft (586.7 m)

Sheet 4 of 5

Depth (ft)	Meters (m)	Profile	Pe	Standard enetration Blows/ft		SPT Blows/6" (N)	Sample Type	Sample No.	Lab	Tests	Driller Joe Judd Description of Material	Groundwater #pir	-
	Σ		10 20	0 30	40	18 (36)	Sar	Sa			Length Recovered 1.0 ft.	Gro	
	- 22			1								-	
75—	-23					13 17 19 (36)	X	D-15			Well graded SAND with silt, dense, grey, moist, Homogeneous. Length Recovered 1.0 ft.		
80-	- 24				>>4	12 22 29 (51)	Y	D-16			Well graded SAND with silt, very dense, brown, moist, Homogeneous. Length Recovered 0.8 ft.		
-	- 25				>>4								
85-	-26					13 23 30 (53)		D-17			Well graded SAND with silt, very dense, grey, moist, Homogeneous. Length Recovered 0.9 ft		
90 -	-27			1		9 15 18 (33)	Y	D-18			Well graded SAND with silt, dense, grey, moist, Homogeneous. Length Recovered 1.0 ft.	-	
	-28				I I I I								
-				1	i	10 14	Y	D-19			Well graded SAND with silt, dense, grey, moist, Homogeneous.	-	



Start Card S-23985

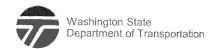
HOLE No. PRK-9-04

Job No. XL-2201

395 SR

Elevation _1924.8 ft (586.7 m)

Depth (ft)	Meters (m)	Profile	10	Standa Penetrat Blows/	ion	E	SPT Blows/6" (N)	Sample Type	Sample No.	(I ube No.)	Lab	Tests	Description of Material	Groundwater	
100	-30					•	20 (34) 13 20 26 (46)	X	D-20				Length Recovered 1.0 ft. Well graded SAND with silt, dense, grey, moist, Homogeneous. Length Recovered 1.0 ft.	-	
	— 31 -												End of test hole boring at 100.5 ft below ground elevation. This is a summary Log of Test Boring. Soil/Rock descriptions are derived from visual field identifications and laboratory test data. Bailed Hole to 96.0 with no recharge.	-	
105	- 32 - 33													-	
110	- 34														
115	- 35														
-	- 36														



Start Card _____ HOLE No. _PRY-1-00___

Job No_XL-2201	SR	395	Elevation	1915.5 ft (583.8 m)
Project_NSLAC - Perry, Market	t and	Fairview Structures		

Sheet __ 1__ of __3_

Driller Joe Lic#___

Site Address ___

Inspector RRR

Start September 25, 2000 Completion September 26, 2000 Well ID# Equipment Morooka MST-1100

Station LR 514+36 Offset 43.7ft Lt. Casing HWT Method HQ casing advance

	631144.3 Spokane		2815187.20	9	Lati	tude	Longitude	
Depth (ft) Meters (m)	Profile	Standard Penetration Blows/ft 10 20 30 40	SPT Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab Tests		Groundwater
5—————————————————————————————————————			7 6 5 (11)	X	D-1	GS MC	Poorly graded SAND, medium dense, brown, moist, homogeneous, (Qf) SP, MC=10% Poorly graded SAND, medium dense, brown, moist, homogeneous, (Qf)	XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX
15—			13 14 17 (31)	X	D-3		Poorly graded SAND, dense, brown, moist, homogeneous, (Qf)	XXXXXXXXXXXXXX
20 6			10	X	D-4			XXX



Start Card _____

Job No. XL-2201 SR 395 Elevation 1915.5 ft (583.8 m)

HOLE No. PRY-1-00

Sheet __2 of __3_

Project NSLAC - Perry, Market and Fairview Structu

		Project	NSLAC	: - Perry, Marke	t and Fairview	Structure	s	T	1	Driller Joe	Lic#_	
	Depth (ft)	Meters (m)	Profile	Stan Penet Blow 10 20	ration	SPT Blows/6" (N)	Sample Type	Sample No.)	Lab	Description of Material	Groundwater	Instrument
		7			+	16 21 (37)		D-5	GS	Well graded SAND, dense, brown, wet, homogeneous, (Qf)		
	25 —	-8				12 16 (28)	X	5-5	MC	SW, MC=21% Well graded SAND, dense, brown, moist, homogeneous, (Qf)		\$ \$
SOIL GDT 12/1/04,13:55:01	30 —	-10				12 15 18 (33)	X	D-6		Poorly graded SAND, dense, brown, moist, homogeneous, (Qf)		
ARVIEW STRUCTURES GPJ SOIL GDT	35 —	— 11				13 17 22 (39)		D-7	GS MC	SP, MC=15% Poorly graded SAND, dense, brown, moist, homogeneous, (Qf)		XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX
SOIL ALL XL-2201 NSLAC - PERRY, MARKET AND FARVIEW STRUCTURES.GPJ	40	—12 —13				8 14 16 (30)		D-8		Poorly graded SAND, dense, brown, moist, homogeneous, (Qf)		
SOIL AL	45					12		D-9			-000	0 c c c c c c c c c c c c c c c c c c c



Start Card _____

Job No. XL-2201

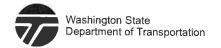
SR 395

Elevation 1915.5 ft (583.8 m)

HOLE No. PRY-1-00

Sheet __3__ of __3__

Depth (ff)	Meters (m)	Profile	10	Stan Penet Blow 20	ration	40	SPT Blows/6" (N)	Sample Type	Sample No.	(Inbe No.)	Lab	Tests	DrillerJoe Description of Material	Groundwater #	
	14			1			14 18 (32)	X					Poorly graded SAND, dense, brown, moist, homogeneous, (Qf)		
50 —	— 15 -				•		15 14 17 (31)	X	D-10	,			Poorly graded SAND with trace of fine gravel, dense, brown, moist, homogeneous, (Qf)	-	XXXX
	— 16				 								End of test hole boring at 51 ft below ground elevation. This is a summary Log of Test Boring. Soil/Rock descriptions are derived from visual field identifications.	-	
55-	- 17												No water table was encountered during drilling. Hole was dry during 5/01 to 3/03		
60	- 18		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1										-	9
	- 19													-	
65	-20				1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1									-	
	- 21				1 1 1 1 1 1										



Start Card S 23723

HOLE No. PRY-2-04

Sheet __1_ of __4__

Project NSLAC - Perry, Market and Fairview Structures

SR

Driller Fetterly Lic# 2708

Site Address Vic. Perry st & proposed SR 395

Inspector Hanning

Start October 5, 2004 Completion October 5, 2004

395

NA _Well ID# __

Elevation 1905.3 ft (580.7 m)

Equipment CME 850 w/ autohammer

Station LR 514+06

Job No. XL-2201

Offset 200.8' Lt.

Casing 3.5"

Method Wet Rotary

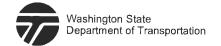
Northing 630989.018

Easting <u>2815225.067</u>

Latitude

Longitude _

Depth (ft)	Meters (m)	Spokar	ne	Subse Standard Penetration	ction N	SPT	Type	e No.	No.)	٩	ts	Section 4 Range 43 EWM Township		nent
Dep	Mete	.:: : :	10	Blows/ft 20 30	40	Blows/6" (N)	Sample Type	Sample No.	(Tube No.)	Lab	Tests	Description of Material	Groundwater	Instrument
T 2/10/05,2:46:59 P2 G			• • • • • • • • • • • • • • • • • • •			3 3 5 (8)	X	D-1				Poorly graded SAND with silt, loose, olive gray, moist, Stratified. Length Recovered 0.8 ft, Length Retained 0.8 ft		
MARKET AND FAIRVIEW STRUCTURES.GPJ SOIL, GDT 2/10/05,2:46:59 P2 C1 C1 C1 C1 C1 C1 C1 C1 C1 C	3					2 3 6 (9)	X	D-2		GS MC		SP-SM, M.C. = 13% Poorly graded SAND with silt, loose, olive gray, moist, Stratified. Length Recovered 0.8 ft, Length Retained 0.8 ft		
SOL ALL AL-2201 INSLAC - PERKY, MARKEL AND PAIRVIEW S	-5					3 6 9 (15)		D-3				Poorly graded SAND with silt, medium dense, olive gray, moist, Stratified. Length Recovered 1.0 ft, Length Retained 1.0 ft		
20-	_ 6		1		ļ ļ								-	



Start Card <u>S 23723</u>

HOLE No. PRY-2-04

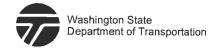
Sheet 2 of 4

Job No. XL-2201

SR 395

Elevation 1905.3 ft (580.7 m)

Depth (ft)	Meters (m)	Profile	Standard Penetration Blows/ft	SPT Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab Tests	Description of Material	Groundwater	1
	-			6 13 21 (34)	X	D-4	GS MC	SP-SM, M.C. = 10% Poorly graded SAND with silt, with some fine gravel, dense, olive gray, moist, Stratified. Length Recovered 1.2 ft, Length Retained 1.2 ft		
	-7			10 13 15 (28)	V A	D-5	GS MC	SP-SM, M.C. = 10% Poorly graded SAND with silt, dense, olive gray, moist, Stratified. Length Recovered 1.0 ft, Length Retained 1.0 ft		
25 —	—8			11 16 17 (33)	V A	D-6		Poorly graded SAND with silt, dense, ofive gray, moist, Stratified. Length Recovered 1.2 ft, Length Retained 1.2 ft		
1	_ 9		•	11 16 17 (33)	V A	D-7	GS MC	SP-SM, M.C. = 19% Poorly graded SAND with silt, dense, olive gray, moist, Stratified. Length Recovered 1.0 ft, Length Retained 1.0 ft		
30 —			,	13 16 17 (33)	X	D-8	GS MC	SP-SM, M.C. = 17% Poorly graded SAND with silt, dense, olive gray, moist, Stratified. Length Recovered 1.0 ft, Length Retained 1.0 ft	-	
74 82	10	7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7		14 13 12 (25)	V	D-9	GS MC	SP, M.C. = 18% Poorly graded SAND, dense, olive gray, moist, Stratified. Length Recovered 1.0 ft, Length Retained 1.0 ft		
35 —	-11			11 14 13 (27)	X	D-10		Well graded SAND, dense, olive gray, moist, Stratified. Length Recovered 1.0 ft, Length Retained 1.0 ft	-	
-	- - 12			10 12 13 (25)	X	D-11	GS MC	SP, M.C. = 16% Poorly graded SAND, dense, olive gray, moist, Stratified. Length Recovered 1.0 ft, Length Retained 1.0 ft		
40 —				10 13 15 (28)	X	D-12		Poorly graded SAND, dense, olive gray, moist, Stratified. Length Recovered 1.0 ft, Length Retained 1.0 ft		
	—13								_	



Start Card S 23723

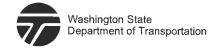
HOLE No. PRY-2-04

Job No. XL-2201 395 SR Elevation 1905.3 ft (580.7 m)

Sheet __3__ of __4_

Project NSLAC - Perry, Market and Fairview Structures

			Perry, Market and Fair						Driller _ Fetterly	Lic#	
Depth (ft)	Meters (m)	Profile	Standard Penetration Blows/ft 10 20 30 40	SPT Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab	Tests	Description of Material	Groundwater	
	14			13 15 16 (31)		D-13			Poorly graded SAND, dense, olive gray, moist, Stratified. Length Recovered 1.0 ft, Length Retained 1.0 ft		
50 —	15 16			13 13 17 (30)	X	D-14			Poorly graded SAND, dense, olive gray, moist, Stratified. Length Recovered 1.0 ft, Length Retained 1.0 ft	-	
55 —	-17			13 15 17 (32)	X	D-15			Poorly graded SAND, dense, olive gray, moist, Stratified. Length Recovered 1.0 ft, Length Retained 1.0 ft	-	
60	— 18 -			12 16 16 (32)		D-16			Poorly graded SAND, dense, olive gray, moist, Stratified. Length Recovered 1.0 ft, Length Retained 1.0 ft		
65 —	-19 -20			15 16 16 (22)		D-17		's	Poorly graded SAND, dense, olive gray, moist, Stratified. Length Recovered 1.0 ft, Length Retained 1.0 ft	-	
	-21			(32)							



Job No. XL-2201

LOG OF TEST BORING

Start Card S 23723

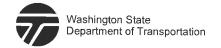
HOLE No. PRY-2-04

SR 395

Elevation __1905.3 ft (580.7 m)

Sheet <u>4</u> of <u>4</u>

22 75 — 23 75 — 24 Poorly graded SAND, dense, olive gray, moist, Stratified. Length Recovered 1.0 ft, Length Retained 1.0 ft Poorly graded SAND, dense, olive gray, moist, Stratified. Length Recovered 1.0 ft, Length Retained 1.0 ft Poorly graded SAND, dense, olive gray, moist, Stratified. Length Recovered 1.0 ft, Length Retained 1.0 ft Poorly graded SAND, dense, olive gray, moist, Stratified. Length Recovered 1.0 ft, Length Retained 1.0 ft Poorly graded SAND, dense, olive gray, moist, Stratified. Length Recovered 1.0 ft, Length Retained 1.0 ft Poorly graded SAND, dense, olive gray, moist, Stratified. Length Recovered 1.0 ft, Length Retained 1.0 ft Poorly graded SAND, dense, olive gray, moist, Stratified. Length Recovered 1.0 ft, Length Retained 1.0 ft Poorly graded SAND, dense, olive gray, moist, Stratified. Length Recovered 1.0 ft, Length Retained 1.0 ft Poorly graded SAND, dense, olive gray, moist, Stratified. Length Recovered 1.0 ft, Length Retained 1.0 ft Poorly graded SAND, dense, olive gray, moist, Stratified. Length Recovered 1.0 ft, Length Retained 1.0 ft Poorly graded SAND, dense, olive gray, moist, Stratified. Length Recovered 1.0 ft, Length Retained 1.0 ft Poorly graded SAND, dense, olive gray, moist, Stratified. Length Recovered 1.0 ft, Length Retained 1.0 ft Poorly graded SAND, dense, olive gray, moist, Stratified. Length Retained 1.0 ft Poorly graded SAND, dense, olive gray, moist, Stratified. Length Retained 1.0 ft Poorly graded SAND, dense, olive gray, moist, Stratified. Length Retained 1.0 ft Poorly graded SAND, dense, olive gray, moist, Stratified. Length Retained 1.0 ft Poorly graded SAND, dense, olive gray, moist, Stratified. Length Retained 1.0 ft Poorly graded SAND, dense, olive gray, moist, Stratified. Length Retained 1.0 ft Poorly graded SAND, dense, olive gray, moist, Stratified. Length Retained 1.0 ft Poorly graded SAND, dense, olive gray, moist, Stratified. Length Retained 1.0 ft Poorly graded SAND, dense, olive gray, moist, Stratified. Length Retained 1.0 ft Po	Depth (ft)	Meters (m)	Profile	10	Standa Penetra Blows	ition s/ft	40	SPT Blows/6" (N)	Sample Type	Sample No.	Lab	Tests	Description of Material	Groundwater #	
Poorly graded SAND, dense, olive gray, moist, Stratified. Length Recovered 1.0 ft, Length Retained 1.0 ft End of test hole boring at 76.5 ft below ground elevation. This is a summary Log of Test Boring, Soli/Rock descriptions are derived from visual field identifications and laboratory test data. Bail/recharge. No water		22						16 17		D-18			Poorly graded SAND, dense, olive gray, moist, Stratified. Length Recovered 1.0 ft, Length Retained 1.0 ft		
This is a summary Log of Test Boring. Soil/Rock descriptions are derived from visual field identifications and laboratory test data. Bail/recharge. No water 25 85 28 -27	75—	- 23		1	 	•	 	17 17	X	D-19			Poorly graded SAND, dense, olive gray, moist, Stratified. Length Recovered 1.0 ft, Length Retained 1.0 ft	-	
25	80	—24				1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1							This is a summary Log of Test Boring. Soil/Rock descriptions are derived from visual field identifications and laboratory test data.		
90-		- 25												-	
	85-	- 26												- - -	
	90	-27												_	
		-28					1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1								



Job No_XL-2201

LOG OF TEST BORING

Elevation 1916.3 ft (584.1 m)

Start Card R 65924

HOLE No. PRY-3-04

Sheet __1 _ of __4__

Driller Kerry Cooper Lic# 2552

Project NSLAC - Perry, Market and Fairview Structures

Site Address Vicinity of Perry and Proposed SR-395

Inspector Cleo Andrews

Start September 29, 2004 Completion September 30, 2004 Well ID# AHN-706

SR

Equipment CME 45 w/ autohammer

Station LR 514+01

395

Offset 100.7' Rt. Casing HQ 3" ID x 80.0'

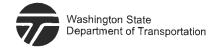
Method Wet Rotary

Northing 631290.314

Easting 2815214.338 Latitude

_____Longitude _

Depth (ft)	Meters (m)	Profile	Stand Penetra Blows	ation	SPT Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab	Tests	Description of Material	Groundwater	Instrument
50	-1 -2		•		3 2 4 (6)	X	D-1			Poorly graded SAND, loose, brown, wet, Homogeneous, HCl reaction not tested, 0.0' to 4.5 Poorly graded Sand as indicated by drilling and wash return. 100% drilling fluid return. Length Recovered 1.5 ft, Length Retained 1.0 ft	1 7	
10 —	-3				8 8 11 (19)	X	D-2			Well graded GRAVEL with sand, subrounded, medium dense, brown, wet, Homogeneous, HCl reaction not tested Length Recovered 1.2 ft, Length Retained 1.0 ft	-	
15 —	-5	O. O.			11 10 12 (22)	X	D-3			Well graded SAND with gravel, medium dense, brown, wet, Homogeneous, HCl reaction not tested Length Recovered 1.2 ft, Length Retained 1.0 ft	- \	
20	-6		•		7	X	D-4			Well graded SAND with gravel, dense, brown, wet,		



Start Card R 65924

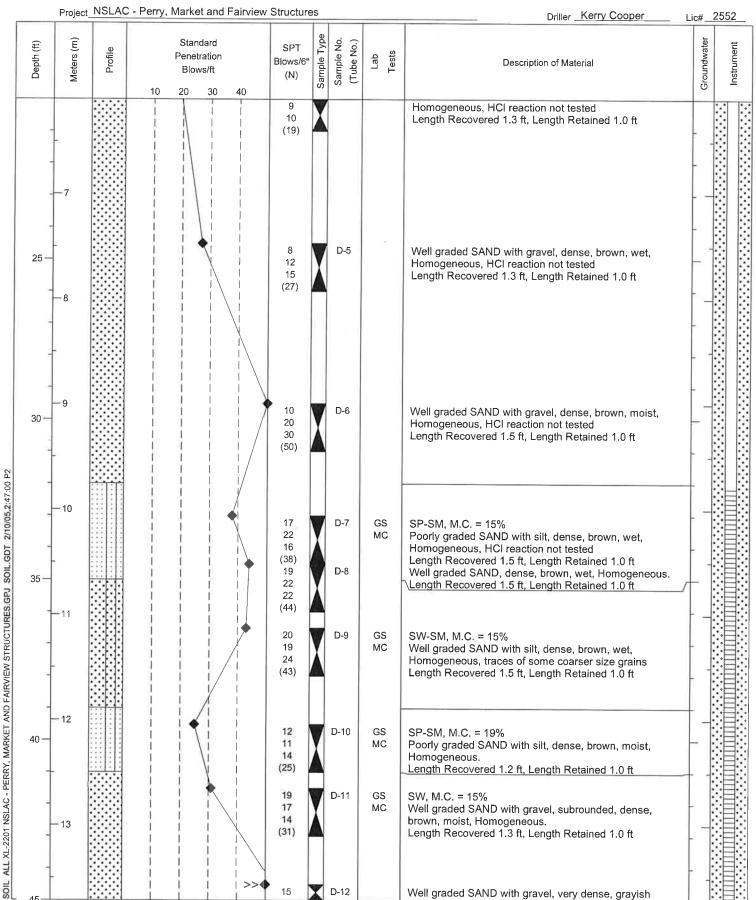
HOLE No. PRY-3-04

Job No. XL-2201 SR

395

Elevation 1916.3 ft (584.1 m)

Sheet __2 of __4





Start Card R 65924

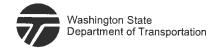
HOLE No. PRY-3-04

Sheet __3 of __4

Job No. XL-2201 SR 395

Elevation <u>1916.3 ft (584.1 m</u>)

Depth (ft)	Meters (m)	Profile	Standard Penetration Blows/ft 10 20 30	40	SPT Blows/6" (N)	Sample Type	Sample No. (Tube No.)	Lab	Tests	Description of Material	Groundwater #	1
-	14 	0 00		 	17 36 (53) 23 23 28 (51)	X	D-13			brown, moist, Homogeneous. Length Recovered 1.5 ft, Length Retained 1.0 ft Well graded SAND with gravel, very dense, grayish brown, moist, Homogeneous. Length Recovered 1.5 ft, Length Retained 1.0 ft		
50 —	 15			>>4	24 28 34 (62)	X	D-14			Well graded GRAVEL with sand, subrounded, very dense, grayish brown, moist, Homogeneous. Length Recovered 1.5 ft, Length Retained 1.0 ft	-	
55	−16				55/6 (55)	×	D-15			Well graded GRAVEL with sand, subrounded, very dense, grayish brown, moist, Homogeneous. Length Recovered 0.2 ft, Length Retained 0.2 ft		***************************************
60	— 18			•	20 20 30 (50)	X X	D-16			Well graded SAND with gravel, dense, grayish brown, moist, Homogeneous. Length Recovered 1.5 ft, Length Retained 1.0 ft		
65—	19 20			>>	22 33 46 (79)	X	D-17			Well graded SAND with gravel, very dense, grayish brown, moist, Homogeneous. Length Recovered 1.5 ft, Length Retained 1.0 ft		
-	- 21			•	28		D-18			Well graded SAND with gravel, dense, grayish brown,		



Start Card R 65924

HOLE No. PRY-3-04

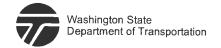
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Job No. XL-2201 SR 395

Elevation 1916.3 ft (584.1 m)

Sheet __4__ of __4__

Depth (ft)	Meters (m)	Profile		Stan Penet Blov	ration vs/ft		SPT Blows/6" (N)	Sample Type	Sample No.	40	Tests		Groundwater	2552
	22		10	20	30	40	17 28 (45)	X				moist, Homogeneous. Length Recovered 1.5 ft, Length Retained 1.0 ft		888888888888888888888888888888888888888
75	- 23			1	1	>>	20 28 32 (60)	V	D-19			Well graded SAND with gravel, very dense, grayish brown, moist, Homogeneous. Length Recovered 1.5 ft, Length Retained 1.0 ft	-	XXXX
-	-		i 1 1	i ! !	į	İ						End of test hole boring at 76 ft below ground elevation.		
	-24			 - - -	1	1						This is a summary Log of Test Boring. Soil/Rock descriptions are derived from visual field identifications and laboratory test data.	-	
80	25 25											(Water table in casing before bailing is 8.0', bailed hole to 72.0', after 51 minutes delay water table at 71.8'. Pulled all casing hole stayed open to 72.0', water table in hole at 48.2'. Installed piezo well at 52.0' with 20.0' screen. After 1 hour delay water table in piezo well is dry. Ended test hole at 76.0' below ground elevation. 9/30/04.		
85—	-26		1		1									
1			 - -		1	i ! !								
	-27		! ! !		1	Î 1 1 1							-	
90-	81		 	1 1 1	1	1							 -	
_	-28			1	1									
-	• =		ļ	į									-	



Elevation 1852.8 ft (564.7 m)

Start Card S 23736

HOLE No. PRY-4-04

Sheet __1_ of __4_

Driller Sean Verlo Lic# 2615

Project NSLAC - Perry, Market and Fairview Structures

SR

Site Address Vic. Perry st & proposed SR 395

Station LR 514+02.03

Start December 28, 2004 Completion December 28, 2004 Well ID# Equipment CME 45 w/ autohammer

Inspector Ybarra

Job No. XL-2201

Offset 46.06' Lt. Casing HW 4.5/HQ 3.5

395

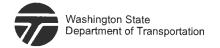
Method Wet Rotary

Northing _631143.7736

Easting 2815221.779 Latitude

_____Longitude _

Depth (ft)	Meters (m)	Profile	Standard Penetration Blows/ft 10 20 30 40	SPT Blows/6" (N)	Sample Type	Sample No.	(Tube No.)	Lab	Description	of Material	Groundwater	
-	•											
	_1										-	3
5-	-2										- ⁻	
-	_									*		
10	-3											
-	-4										-	
15—											-	
	-5									12/28/2004	Ž	
~		đi.										



Start Card S 23736

HOLE No. PRY-4-04

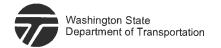
Sheet __2_ of __4_

Job No. XL-2201 395 SR

Elevation 1852.8 ft (564.7 m)

Project NSLAC - Perry, Market and Fairview Structures

Depth (ft)	Meters (m)	Profile	Pene	dard tration vs/ft 30 40	SPT Blows/6" (N)	Sample Type	Sample No.	Lab	Tests	Description of Material	Groundwater #2i7	
-											-	
	-7											
25	-8										-	
-												
30 —	-9				16 18 18 (36)	X	D-1			Well graded SAND with gravel, angular, dense, gray, moist, Homogeneous. Length Recovered 1.5 ft, Length Retained 1.5 ft	-	
-	-10										-	
35 —					17 21 23 (44)	Y	D-2			Well graded SAND with gravel, angular, Slightly silty, dense, gray, moist, Homogeneous. Length Recovered 1.5 ft, Length Retained 1.5 ft		
-	-11											
40-	-12			 	12 14 19	Y	D-3			Well graded SAND with gravel, angular, Slightly silty, dense, gray, moist, Homogeneous. Length Recovered 1.0 ft, Length Retained 1.0 ft	-	
					(33)					5, <u></u>	-	
-	-13				19 21 20 (41)	V	D-4			Well graded SAND with gravel, angular, Slighlty silty, dense, gray, moist, Homogeneous. Length Recovered 1.5 ft, Length Retained 1.5 ft	_	
45			1 1		15 16	X	D-5			Well graded SAND with gravel, angular, Slightly silty, dense, gray, moist, Homogeneous.		



Job No_XL-2201

LOG OF TEST BORING

Start Card S 23736

HOLE No. PRY-4-04

Elevation 1852.8 ft (564.7 m)

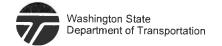
395 SR

Sheet __3__ of __4__

Project NSLAC - Perry, Market and Fairview Structures Driller Sean Verlo Lic# 2615 Sample Type Standard Sample No. Meters (m) (Tube No.) Depth (ft) SPT Profile Instrument Tests Penetration Lab Blows/6' Description of Material Blows/ft (N) 17 Length Recovered 1.0 ft, Length Retained 1.0 ft (33)14 Well graded SAND with gravel, angular, Slightly silty, 15 D-6 17 dense, gray, moist, Homogeneous. Length Recovered 1.3 ft, Length Retained 1.3 ft 15 (32)15 16 D-7 Well graded SAND with gravel, angular, Slightly silty, 18 dense, gray, moist, Homogeneous. 50 17 Length Recovered 1.0 ft, Length Retained 1.0 ft (35)16 Well graded SAND with gravel, subrounded, Slightly silty, 15 dense, gray, moist, Homogeneous. Gravel as indicated by drilling process. 55 19 (34)Length Recovered 1.2 ft, Length Retained 1.2 ft ALL XL-2201 NSLAC - PERRY, MARKET AND FAIRVIEW STRUCTURES.GPJ SOIL.GDT 2/10/05.2:47:01 P2 -18 Well graded SAND with gravel, subangular, Slightly silty, 14 D-9 14 dense, gray, moist, Homogeneous, Large gravel as 60 16 indicated by drilling process. Length Recovered 1.0 ft, Length Retained 1.0 ft (30)19 25 D-10 Well graded SAND with gravel, angular, Slightly silty, 29 very dense, gray, moist, Homogeneous. 28 Length Recovered 1.2 ft, Length Retained 1.2 ft (57)20 21 26 D-11 Well graded SAND with gravel, angular, Slightly silty,

very dense, gray, moist, Homogeneous.

31



Start Card S 23736

HOLE No. PRY-4-04

Job No. XL-2201

28

95

395

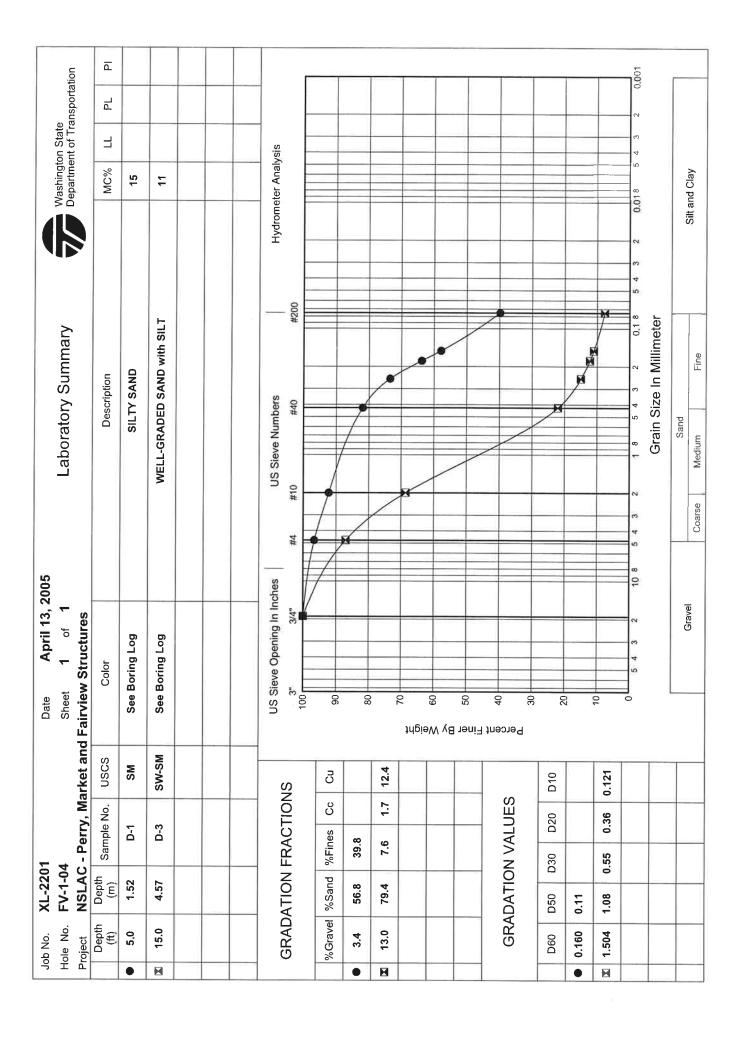
Elevation _1852.8 ft (564.7 m)

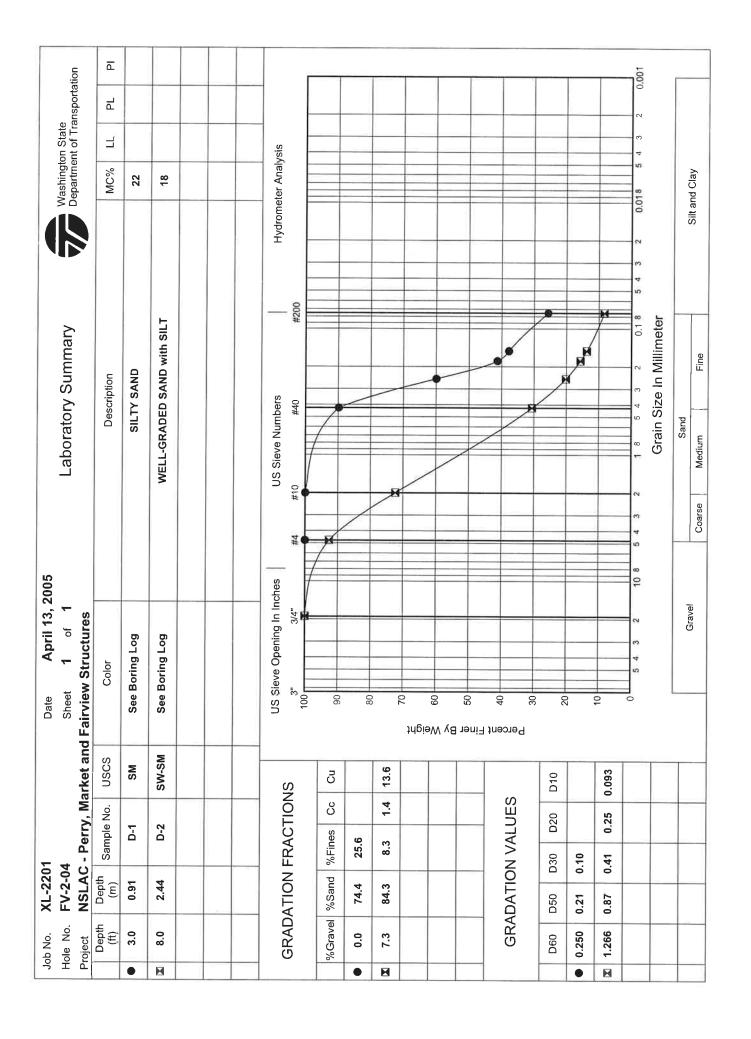
Sheet __4__ of __4__ Project NSLAC - Perry, Market and Fairview Structures Driller Sean Verlo Lic# 2615 Sample Type Meters (m) Standard Sample No. Groundwater Depth (ft) (Tube No.) SPT Profile Instrument Tests Penetration Lab Blows/6" Description of Material Blows/ft (N) 20 30 35 Length Recovered 1.0 ft, Length Retained 1.0 ft (66)-22 >>◀ D-12 36 No Recovery 43 75 50/3 (93)23 End of test hole boring at 75.3 ft below ground elevation. This is a summary Log of Test Boring. Soil/Rock descriptions are derived from visual field identifications and laboratory test data. 24 80 SOIL ALL XL-2201 NSLAC - PERRY, MARKET AND FAIRVIEW STRUCTURES.GPJ SOIL.GDT 2/10/05,2:47:01 P2 25 85 -26 27 90

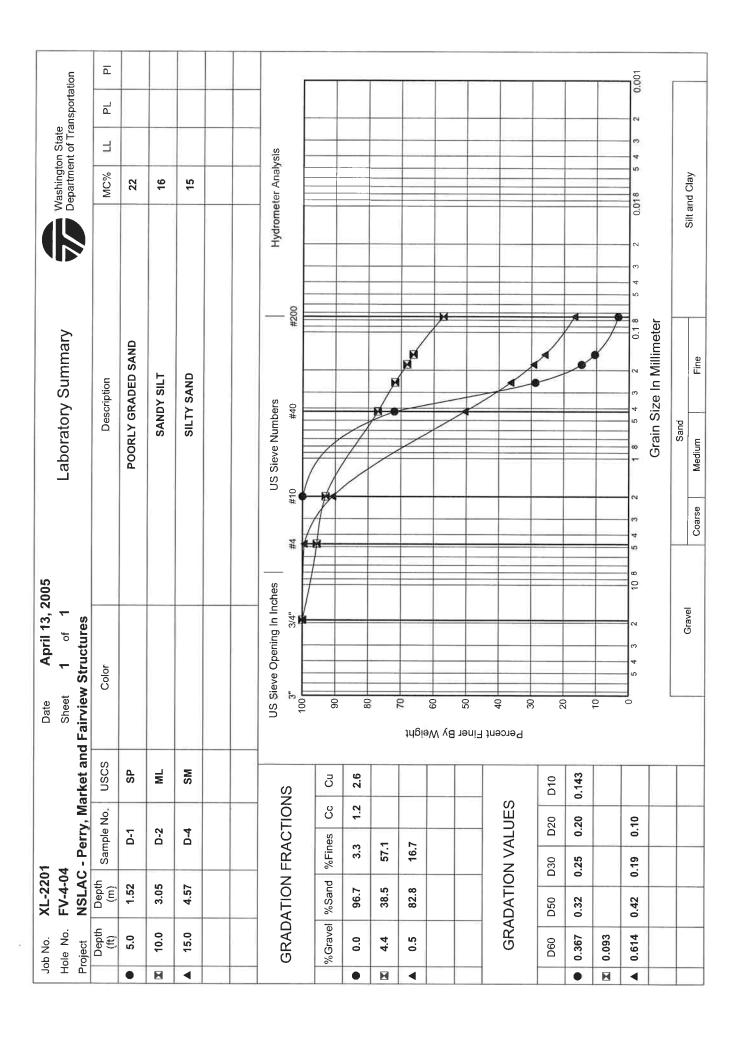
APPENDIX C - LABORATORY TESTING

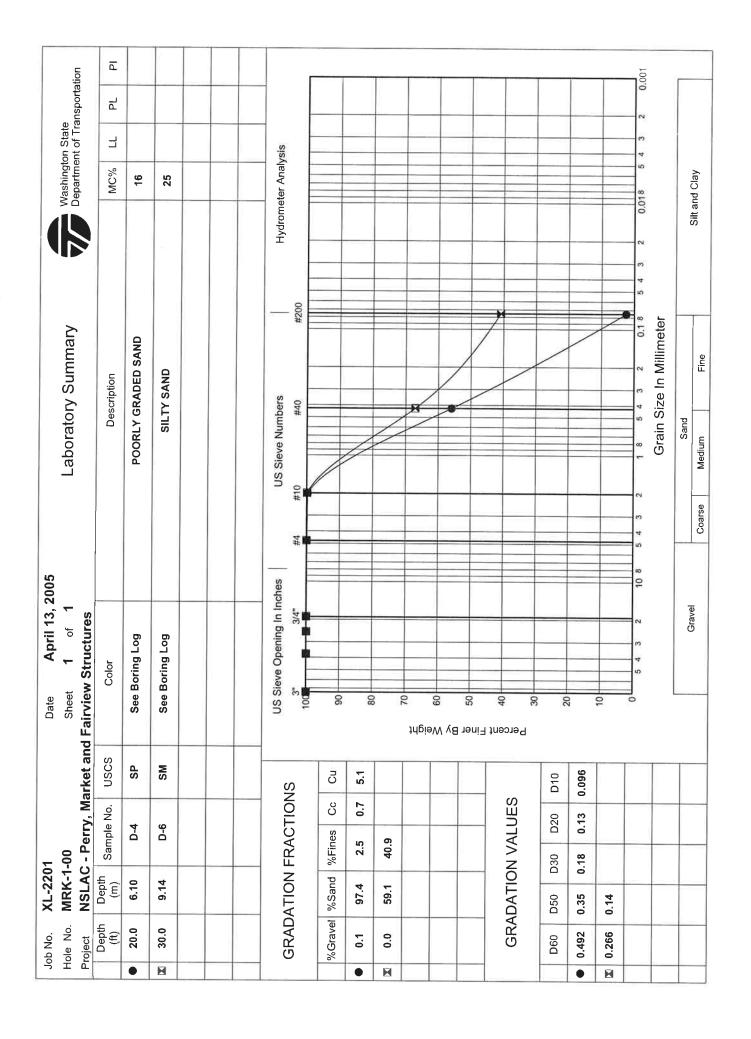
LABORATORY TESTING

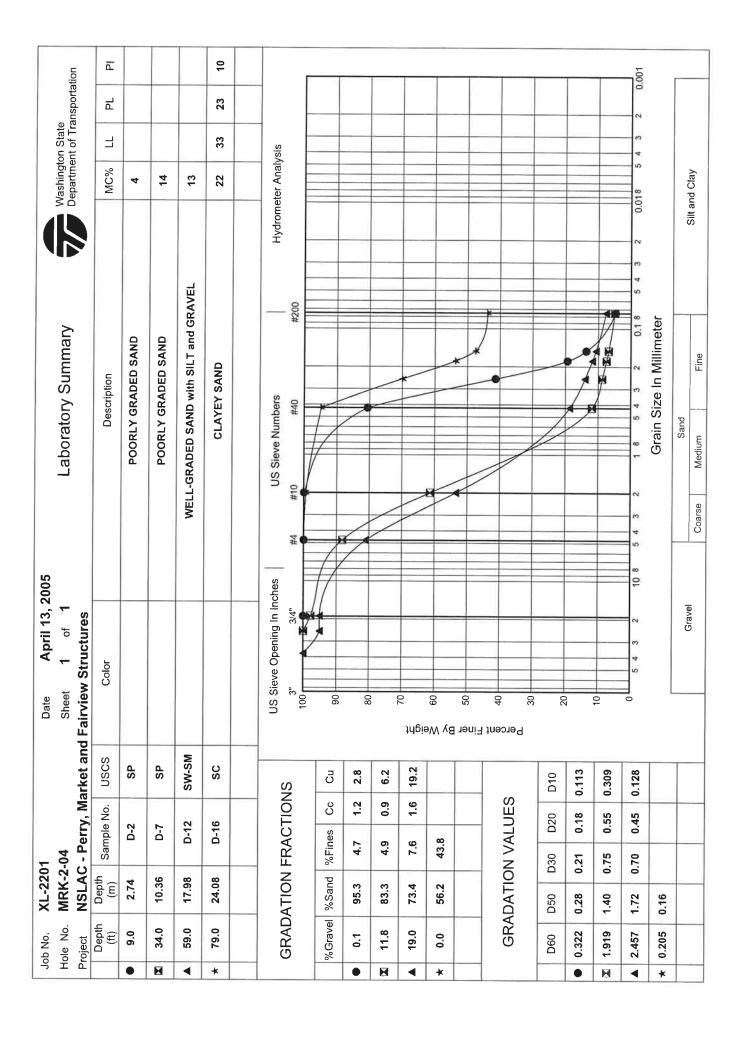
Laboratory testing was performed on selected samples from the field exploration program. Testing included performing moisture content, grain size analyses and Atterberg Limits. The tests were done in general accordance with AASHTO T-88, T-89, and T-90 guide specifications, respectively. After the testing was complete, the samples were classified in general accordance with the Unified Soil Classification System (USCS).

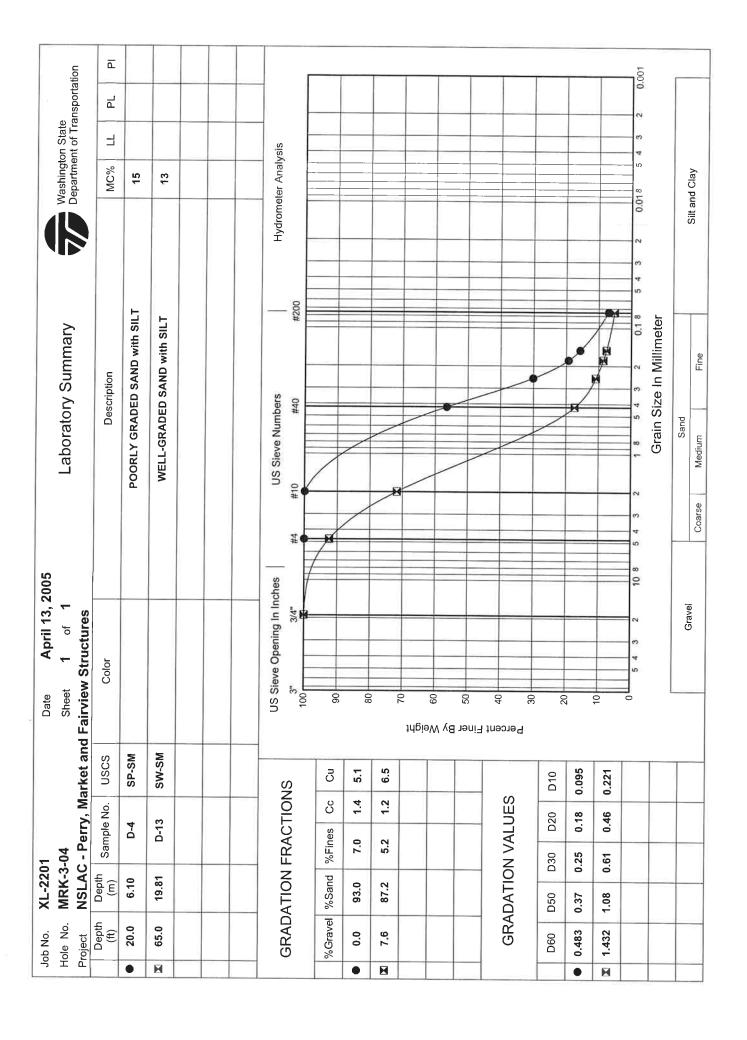








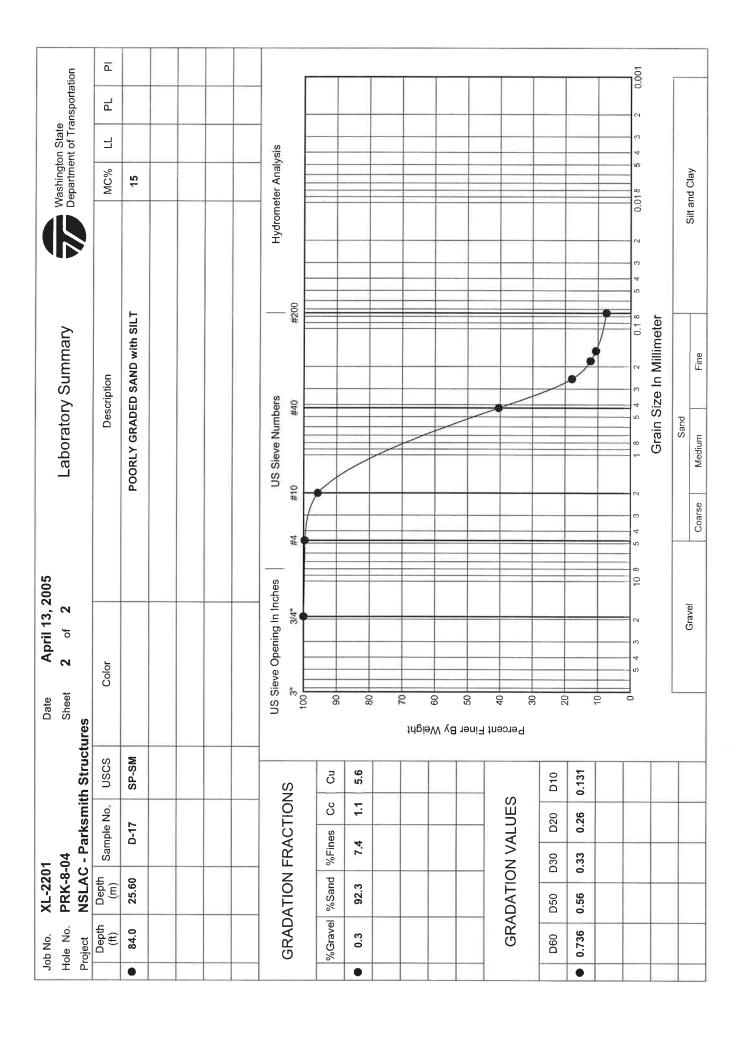




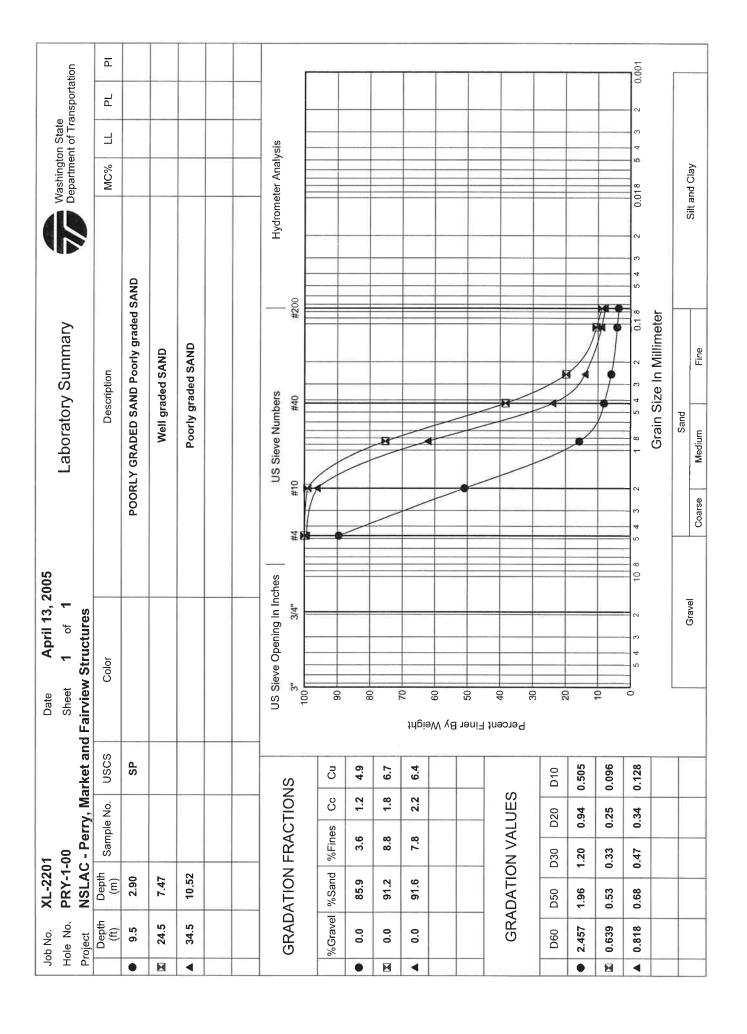
ヹ む	Job No. Hole No. Project	MRK-4-04 NSLAC - F	-04 - Pern	y, Mari	MRK-4-04 NSLAC - Perry, Market and Fairview Structures	Date Sheet airvie	Api 1	April 13, 2005 I of 1 uctures	10		Laboratory Summary	tory Su	ımmar	_	113	Washi	Washington State Department of Transportation	tate	ortation
	(#)	E E	San	-	nscs		Color					Description				WC%	// // // // // // // // // // // // //	- PL	<u>a</u>
•	34.0	10.36	D-7		SW-SM					>	WELL-GRADED SAND with SILT	DED SAN	D with S	1		14	_		
H	39.0	11.89	8-0		SW-SM					>	WELL-GRADED SAND with SILT	DED SAN	D with SI	5		6			
4	79.0	24.08	D-16		SW-SM					5	WELL-GRADED SAND with SILT	DED SAN	D with SI	ا ا دا		10			
_	GRAD,	ATION	GRADATION FRACTIONS	TIONS	(0	US Si.	eve Open	US Sieve Opening In Inches 3/4"	¥•	#10	US Sieve Numbers	mbers #40		#200	f	Hydrometer Analysis	nalysis		-
Ť	%Gravel	%Sand	%Fines	ပိ	- To	06													
•	0.0	91.0	0.6	5.	7.4	C C				_									
M	7.8	84.6	7.5	1.8	11.0	8 6													
◀	14.7	78.4	6.8	1.1	17.7							Je -							T
						8 8													
						riner i						•							
	GRAI	DATIO	GRADATION VALUES	JES	·	Percent 8													
	090	D20 D	D30 D2	D20 D	D10	Č													
•	0.622	0.46 0	0.28 0.2	0.20 0.0	0.084	07						4 14							T
	1.490	1.10 0	0.60 0.4	0.45 0.1	0.136	10								77					
4	2.293	1.54 0.	0.56 0.31		0.129		5 4 3	2 10	8 5 4	3	- 60	5 4 3	2 0.	0.18 5	4 3 2	0.018	5 4	- 5	J.00
-						ļ					Grain	Size In	Grain Size In Millimeter	е					
-					Γ			Grave			Sand	ַ				100			
\dashv								Gava	් <u>පි</u>	Coarse	Medium	_	Fine	Ī		Silt and Clay	Ŋ		

ゔ :	Job No.	XL-2201	- 3			Date	April 13, 2005			1	_ ≥	Washingtor	Washington State
ΞŒ	Hole No. Project	PRK-7-04 NSLAC -	-04 } - Parks	smith	PRK-7-04 NSLAC - Parksmith Structures	Sheet es	1 of 1	Laboratory Summary	immary 🔨		۵	Departmen	Department of Transportation
	Depth (ft)	Depth (m)	Sample No.	No.	nscs		Color	Description				MC%	MC% LL
•	9.0	2.74	D-2		SM			SILTY SAND	Q			=	11
M	14.0	4.27	D-3		SW-SM			WELL-GRADED SAND with SILT	D with SILT			91	16
•	24.0	7.32	D-5		SM			SILTY SAND	Q		~	26	9
*	39.0	11.89	8-Q		SP-SM			POORLY GRADED SAND with SILT	ND with SILT		7	22	2
		Į Ž				US Sie	US Sieve Opening In Inches	US Sieve Numbers		Hydrometer Analysis	er /	Analys	Analysis
	GRAD		GRADA LIOIN FRACTIONS	020	•	100 H	3/4"	#4 #10 #40	#200				
	%Gravel	%Sand	%Fines	రి		06							
•	4.0	80.5	15.5			80		•					
×	3.7	90.2	6.2	9.1	6.1								
4	0.0	79.4	20.6										
*	0.1	88.4	11.5	1.5	4.4	& &					-		
						SO SO			4				
	GRA	DATIO	GRADATION VALUES	JES		Percent 5							
	D90	D50 [D30 D2	D20 D	D10	5 8							
•	0.886	0.63	0.22 0.1	0.10		3		1					
×	1.082	0.87	0.55 0.4	0.44 0.7	0.179	0			* M			-	
◀	0.184	0.16 0	0.10			-	5 4 3 2 10 8	5 4 3 2 1 8 5 4 3	2 0.18 5 4 3 2	2 0.018		υ.	5 4 3
*	0.306	0.26 0	0.18 0.1	0.12		Į		Grain Size In Millimeter	Millimeter				
				-			Grave	-		Silt and Clay	֟֟֟֟֟֟ <u>֚</u>	 	2
				_			240	Coarse Medium	Fine	S 110	d Clay		

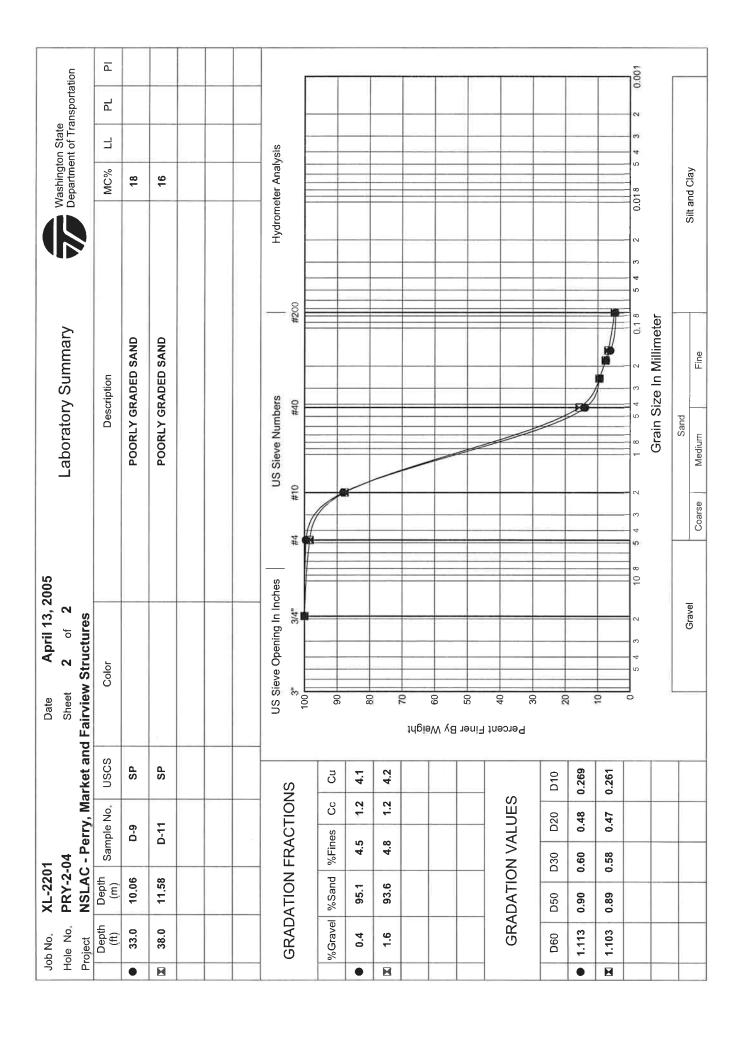
4		-//- ×				1										
JOD 140.		VL-2201	-			טמנו		-	-	4 4 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	!		Washing	oton State	4	
Hole	Hole No.	PRK-8-04	-04 -			Sheet	1 of	5	_	Laboratory Summary	ımmary		7 Departing	Department of Transportation	ansporta	tion
Project		NSLAC	NSLAC - Parksmith Structures	smith \$	Structu	res										
_	Depth (ft)	Depth (m)	Sample No.		nscs		Color			Description	Ē		WC%	=======================================	Ъ	₫
•	4.0	1.22	D-1		SP-SM				PO	POORLY GRADED SAND with SILT	AND with SILT		15			
	14.0	4.27	D-3		ML					SANDY SILT	LT		32			
4	24.0	7.32	D-5		SW-SM				8	WELL-GRADED SAND with SILT	VD with SILT		14			
*	29.0	8.84	9-Q		SM					SILTY SAND	Ę.		20			
•	49.0	14.94	D-10		SW-SM				3	WELL-GRADED SAND with SILT	ID with SILT		12			
۳	;RAD	ATION	GRADATION FRACTIONS	TIONS		US Siev	/e Openir	3/4"	#4 #10	US Sieve Numbers	#200		Hydrometer Analysis	alysis	-	Г
%	%Gravel	%Sand	%Fines	3	n n	06		4			H H					
•	1.5	92.9	5.6	1.4	4.9	80										Т
	0.0	31.9	68.1			ř.			/		<i>*</i>					
4.	0.0	89.9	10.1	2.1	6.1				<i>y</i> *		**					
*	0.0	6.62	20.1			By M										Т
·	13.8	79.4	8.9	4.1	10.7	Finer S					*					1
	GRA	DATIO	GRADATION VALUES	UES		Percent 8 &										
_	D90	D50 [D30 D	D20 D	D10	20										— T
-	1.074	0.87	0.57 0.	0.46 0.2	0.219	í s									-	-
						0.										
•	0.450	0.37 (0.26 0.	0.19		0	5 4 3	2 10 8	5 4 3 2	18543	2 0.18	ε 2 2 3	2 0.018	5 4	- 67	0.00
*	0.212	0.19	0.11							Grain Size In Millimeter	n Millimeter					Γ
0	1 796	1 28	0 64	0.46 0.1	0 167			Gravel	(Sand	i		Silt and Clay	>		
_	_	-	-	_	2				Coarse	Medium	Fine					_



NSIAGO-Parksmith Structures NSIAGO-Parksmith Structures Description NSIAGO Serve Opening in Inches SIATY SAND SIATY SAN	ゔ —	Job No.	XL-2201	Ξ ;			Date		April 13, 2005					4	1		3
140 Paph P	Ι Δ	Hole No. Project	PRK-9 NSLAC	-04 > - Park	smith	Structu	Sheet	1 of	-		Laborator	y Summary		Dek Ma Dek	shingtor artmen	_ +	n State t of Tran
14.0 4.27 D-3 SW-SM		Depth (ft)	Depth (m)	Sample	o N O	nscs		Color			Desc	ription			VC%		=
14.0 4.27 D-3 SM SM SM SM SM SM SM S	•	9.0	2.74	D-2		SP-SM				1	OORLY GRADE	D SAND with SI	-1		20		
13.0 5.79 D-4 SW-SM 34.0 10.36 D-7 SM 34.0 10.36 D-7 SM 34.0 10.36 SS-3 1.3 4.4 35.0 SS-3 SS-3 SS-3 35.0	M	14.0	4.27	D-3		SM					SILTY	SAND			15	1.	
340 10.36	•	19.0	5.79	D-4		SW-SM					WELL-GRADED	SAND with SIL			20		
Caral State Numbers Caral Cara	*	34.0	10.36	D-7		SM					SILTY	SAND			21	0 1	
GRADATION FRACTIONS Contract									-								
%Gravel %Sand %Fines CC Cu 24 92.3 5.3 1.3 4.4 0.0 87.8 12.2 1.9 4.7 0.0 86.5 13.5 1.3 4.7 0.0 86.5 13.5 1.0 50 0.0 86.5 13.5 1.0 50 0.0 86.5 13.5 1.0 50 0.0 86.5 13.5 1.0 50 0.0 86.5 13.5 1.0 1.0 0.0 86.5 13.6 1.0 1.0 0.0 1.0 1.0 1.0 1.0 0.0 1.0 1.0 1.0 1.0 0.0 1.0 1.0 1.0 1.0 0.0 1.0 1.0 1.0 1.0 0.0 1.0 1.0 1.0 1.0 0.0 1.0 1.0 1.0 1.0 0.0 <td< td=""><td></td><td>GRAD</td><td>ATION</td><td>FRAC</td><td>TIONS</td><td>(0</td><td>US S</td><td>ieve Opening</td><td>In Inches</td><td>#10</td><td>JS Sieve Numbe</td><td></td><td></td><td>Iromete</td><td>r Analysis</td><td>10</td><td></td></td<>		GRAD	ATION	FRAC	TIONS	(0	US S	ieve Opening	In Inches	#10	JS Sieve Numbe			Iromete	r Analysis	10	
CRADATION VALUES 0.0 86.5 13.5 1.3 4.4 0.0 86.5 13.5 1.3 4.3 0.0 86.5 13.5 0.0 86.5 13.5 0.0 86.5 13.5 0.0 86		, o		0 / L			3										
2.4 92.3 5.3 1.3 4.4 0.0 86.5 13.5		%Gravel		%rines	3	3	06										
0.0 86.5 13.5 1.9 4.7 0.0 86.5 13.5 1	•	2.4	92.3	5.3	1.3	4.4	80				9 *						
0.0 86.5 13.5 Section 1.8 6.7 Section 1.8 6.7 Section 1.8 6.7 Section 1.8 6.7 Section 1.9 M	0.0	87.8	12.2	1.9	4.7	F				/							
GRADATION VALUES GRADATION VALUES Dec Dso	•	0.1	89.3	10.6	1.8	6.7											
GRADATION VALUES DEC DE	*	0.0	86.5	13.5													
GRADATION VALUES D60 D50 D50 D20 D10 1.027 0.84 0.56 0.46 0.234 0.303 0.27 0.19 0.13 0.309 0.27 0.19 0.11 Sand Grain Size In Millimeter Sand																	
1.027 0.84 0.56 0.46 0.234 0.303 0.27 0.19 0.11 0.309 0.27 0.19 0.11 Sand		GRA	DATIO	N VALI	UES			-1									
1.027 0.84 0.56 0.46 0.234 10 0.13 10 0.303 0.27 0.19 0.13 10 0.309 0.27 0.19 0.11 Sand		D90		-	-	710	5				/						
0.303 0.27 0.19 0.13 0.479 0.37 0.25 0.16 0.309 0.27 0.19 0.11 0.309 0.27 0.19 0.11 0.309 0.27 0.19 0.11 0.309 0.27 0.19 0.11 0.309 0.27 0.19 0.11 0.309 0.27 0.19 0.11	•	1.027			-	234	07						*				
0.479 0.37 0.25 0.16 0 0 5 4 3 2 10 8 5 4 3 2 1 8 5 4 3 2 0.18 5 4 3 2 0.018 5 6 0.30 0.27 0.19 0.11 Grain Size In Millimeter	×	0.303			13		10						4 9				
0.309 0.27 0.19 0.11 Grain Size In Millimeter	▲	0.479	-		16		10	ω	10 8	e 4	8	3	8 5 4 3	0.018	- 6	6	~
Sand	×	0.309	-	-	=						Grain Si;	ze In Millimete	.				
					+			ن ا	jova		Sand						



Hotel Toj	Hole No.		04			Date	∢	April 13, 2005	005						4			
Proj		PRY-2-04				Sheet	•	of 2			Laborate	Laboratory Summary	lary	r	Washington StateDepartment of Transportation	ton State ent of Tr	ansp	ř.
	Project	NSLAC	: - Perry	', Mark	NSLAC - Perry, Market and Fairview Structures	airvie	w Stru	ctures										:
	Depth (ft)	Depth (m)	Sample No.		nscs		Color				ă	Description			WC%	=======================================	4	
•	10.0	3.05	D-2		SP-SM					_	POORLY GRADED SAND with SILT	DED SAND w	ith SILT		13			
	20.0	6.10	D-4		SW-SM						WELL-GRAD	WELL-GRADED SAND with SILT	h SILT		10			
4	23.0	7.01	D-5		SP-SM					_	POORLY GRADED SAND with SILT	DED SAND w	ith SILT		9			
*	28.0	8.53	D-7	S	SP-SM						POORLY GRADED SAND with SILT	DED SAND w	ith SILT		19			
•	30.0	9.14	D-8		SP-SM					<u>i-</u>	POORLY GRADED SAND with SILT	DED SAND w	ith SILT		17			
0	3RAD/	\TION	GRADATION FRACTIONS	SNOL		s sn	Sieve Op	US Sieve Opening In Inches	_))	US Sieve Numbers	nbers #40		Η	Hydrometer Analysis	lysis		
						100	E					04#	#200				-	- 1
%	%Gravel	%Sand	%Fines	ပိ	n _O	06		4									+	
•	1.3	91.9	6.8	1.5 5	5.4	80												
H	5.0	86.0	9.0	1.2 9	9.1	ř				*	1							
4	8.0	9.98	5.4	1.2	75.													1
×	6.0	92.6	6.4	1.5	5.4	By W											-	1
•	0.7	93.6	5.7	1.4	5.0	Teni-H											_	- 1
	GRA	DATIO	GRADATION VALUES	JES	,	Percent 6						8						
	090	D50 [D30 D2	D20 D1	D10	20						×						
•	1.010	0.82 0	0.54 0.4	0.44 0.1	0.188	3												
•	0.847	0.55 0	0.31 0.2	0.23 0.0	0.093	10												1
-	1.298	1.01	0.61 0.4	0.48 0.2	0.238	0	رن 4	3	10 8	5 4 3 2	8	5 4 3 2	0.18	5 4 3 2	0.018	5 4	- 2	0.001
*	0.897	0.72 0	0.47 0.3	0.33 0.1	0.166						Grain	Grain Size In Millimeter	imeter					
0	0.978	0.79	0.52 0.4	0.42 0.1	0.194			Gravel			Sand	- D			Silt and Clay			

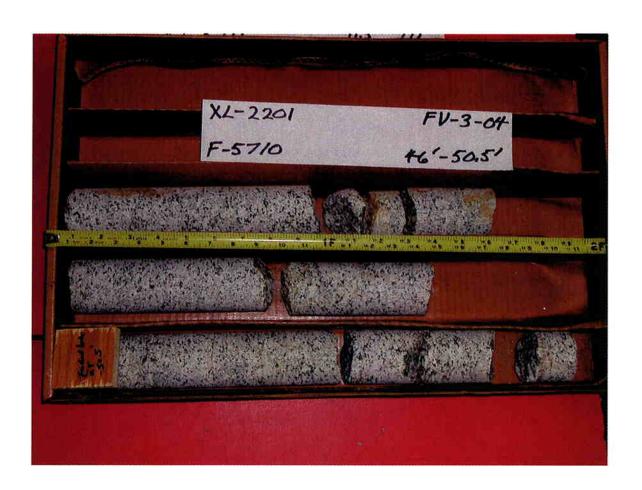


ન	Job No.	XL-2201	Ξ			Date April 13, 2005				
Ĭ	Hole No.	PRY-3-04	.04				Laboratory Summary	Washington State	tate	1
٦	Project	NSLAC	} - Perry	/, Mark	ket and	v Struct		Department of	ransp	ortallo
	Depth (ft)	Depth (m)	Sample No.		nscs	Color	Description	MC% LL		చ
•	33.0	10.06	D-7		SP-SM		POORLY GRADED SAND with SILT	15		
×	36.5	11.13	6-Q		SW-SM		WELL-GRADED SAND with SILT	15		
4	39.5	12.04	D-10		SP-SM		POORLY GRADED SAND with SILT	19		
*	41.5	12.65	D-11		SW		WELL-GRADED SAND with GRAVEL	15		
						US Sieve Opening In Inches	US Sieve Numbers	Hydrometer Analysis	-	
	GRAD	ATION	GRADATION FRACTIONS	TIONS	(0	100 1	#4 #10 #40 #200			
	%Gravel	%Sand	%Fines	3	J.	06				
•	8.0	93.9	5.3	1.3	8.4	80				
M	0.2	91.8	8.0	2.2	8.2	C F				
4	3.1	8.06	6.1	1.3	5.4		*			
*	20.1	75.1	4.8	1.1	8.4	By We				
						Finet 50				
	GRA	DATIO	GRADATION VALUES	UES		Percent 4 8				
	D90	D50 [D30 D2	D20 D	D10	30	R			
•	1.111	0.89	0.57 0.4	0.46 0.2	0.231	}				
M	0.944	0.76	0.48 0.3	0.33 0.1	0.115					
•	0.879	69.0	0.43 0.3	0.30 0.1	0.162	0 5 4 3 2 1	108 54 3 2 18 54 3 2 0.18 54 3 2	0,018 5 4	3 2	0.001
*	1.799	1.27 0	0.64 0.4	0.45 0.2	0.215		Grain Size In Millimeter			
1				-		Javer	Sand	Silt and Clay		
_						2002	Coarse Medium Fine	all aliu ciay		

APPENDIX D – ROCK CORE PHOTOS













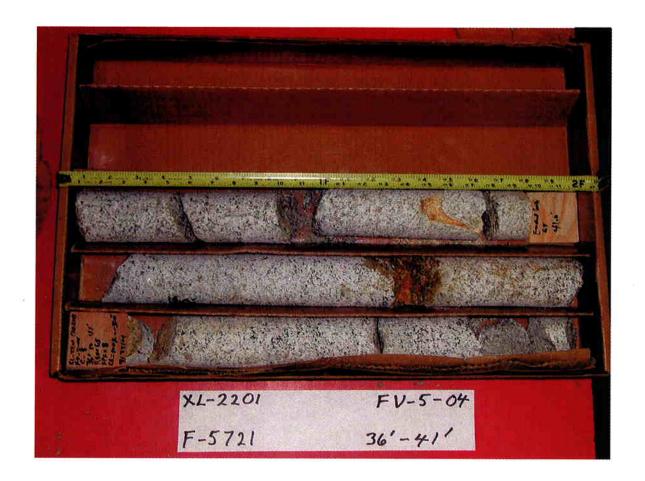


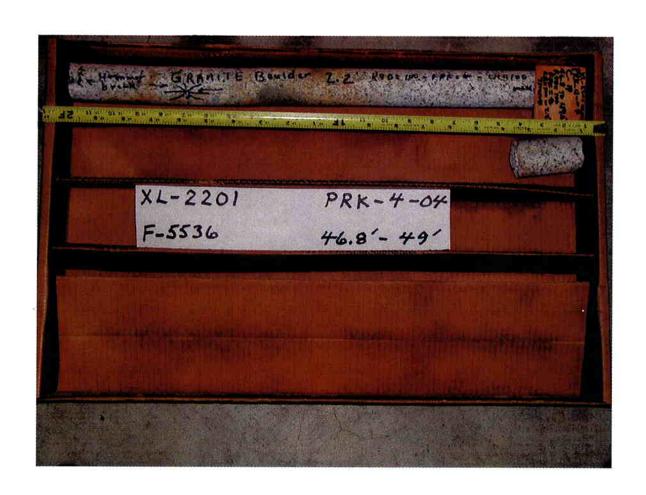












APPENDIX E – BRIDGE FOUNDATION DESIGN INFORMATION

SR 395 Northbound Overcrossing at Fairview Road

Pier 1 Bearing Capacity

Maximum Footing Elevation: 2056'

Soils at Pier 1 should be excavated to the bedrock and replaced with Gravel Borrow, Method C compaction, to elevation 2056'. If bedrock is encountered at or above the footing elevation, the rock should be removed to 1 foot below the footing elevation and replaced with Gravel Borrow, Method C compaction.

Footing Width (ft)	Ultimate Bearing Capacity (ksf)	Service Bearing Capacity (ksf) Based on 1" of Settlement*
6	58.4	23.0
9	71.3	17.5
12	94.3	14.7
15	97.0	13.5
18	109.3	12.2
21	121.1	11.5

^{*} Due to difference in elevation of bedrock, the service limit state bearing capacity is based on 1 inch of settlement at the west side of the abutment and 3/8 of an inch at the east side of the abutment.

Pier 2 Bearing Capacity

Maximum Footing Elevation: 2050'

Footing Width (ft)	Ultimate Bearing Capacity (ksf)	Service Bearing Capacity (ksf) Based on 1" of Settlement
6	17.3	11.5
9	20.2	9.2
12	23.3	8.2
15	26.4	7.5
18	29.4	7.2
21	32.2	7.0

SR 395 Northbound Overcrossing at Fairview Road (cont.)

Lateral Earth Pressure Coefficients and Soil Parameters

Parameter	Value
Backfill Unit Weight (γ)	130 pcf
Backfill Soil Friction Angle (φ _f)	36°
Active Earth Pressure (K _a)	0.26
At Rest Earth Pressure (K ₀)	0.41
Bearing Soil Friction Angle (ϕ_f)	36° at Pier 1, 29° at Pier 2
Passive Earth Pressure (K _p) - Unfactored	3.85
Coefficient of Sliding	0.73 at Pier 1, 0.55 at Pier 2
Seismic Coefficient (Kae)	0.28

Shear Modulus versus Foundation Soil Strain

Pier 1

Shear Modulus, G	Strain	Poisson's ratio, μ
500 to 1500 ksf	0.2 to 0.02 %	0.35

Pier 2

Shear Modulus, G	Strain	Poisson's ratio, μ
280 to 835 ksf	0.2 to 0.02 %	0.35

SR 395 Northbound Overcrossing at Parksmith Drive/BNSF

Pier 1Bearing Capacity

Maximum Footing Elevation: 1930'

Footing Width (ft)	Ultimate Bearing Capacity (ksf)	Service Bearing Capacity (ksf) Based on 1.5" of Settlement
6	60	19
9	69	13.5
12	76	10.5
15	83	9.0
18	91	7.5
21	99	7.0

Piers 2 and 3 Bearing Capacity

Pier 2 Maximum Footing Elevation: 1917'

Pier 3 Maximum Footing Elevation*: 1930'

^{*} Requires embankment construction from existing ground elevation to footing elevation. See Figure 10 in Appendix E.

Footing Width (ft)	Ultimate Bearing Capacity (ksf)	Service Bearing Capacity (ksf) Based on 1.5" of Settlement
6	25.0	11.5
9	30.0	8.25
12	35.0	6.75
15	40.0	6.0
18	45.0	5.25
21	50.0	4.75

SR 395 Northbound Overcrossing at Market Street

Pier 1 Bearing Capacity

Ri-				
Footing	Ultimate	Ultimate	Service Bearing	Service Bearing
Width (ft)	Bearing	Bearing	Capacity (ksf)	Capacity (ksf)
	Capacity (ksf)	Capacity (ksf)	Based on 2" of	Based on 2" of
	Footing at or	Footing at	Settlement	Settlement
	below elevation 1950'	approx. elevation 1960' *	Footing at or below elevation 1950'	Footing at approx. elevation 1960' *
6	19.5	7.9	16.0	20.0
9	23.1	11.9	11.0	14.0
12	26.8	15.8	9.0	11.0
15	30.5	19.8	8.0	9.0
18	34.3	23.7	7.0	8.0
21	37.9	27.7	6.5	7.5

^{*} Footing elevation at approximately elevation 1960' assumes the footing is placed within the embankment fill and meets the requirements of Figure 9.5.1-1 of the BDM.

Pier 2 Bearing Capacity

Maximum Footing Elevation: 1950'

Footing Width (ft)	Ultimate Bearing Capacity (ksf)	Service Bearing Capacity (ksf) Based on 2" of Settlement
6	19.5	13.0
9	23.1	10.0
12	26.8	8.0
15	30.6	7.0
18	34.4	6.25
21	38.1	5.75

SR 395 Northbound Overcrossing at Market Street (cont.)

Lateral Earth Pressure Coefficients and Soil Parameters

Pier 1, Footing at approx. elev. 1960'

Parameter	Value
Backfill Unit Weight (γ)	130 pcf
Backfill Soil Friction Angle (φ _f)	36°
Active Earth Pressure (K _a)	0.26
At Rest Earth Pressure (K ₀)	0.41
Bearing Soil Friction Angle (\$\phi_f\$)	36°
Passive Earth Pressure (K _p) - Unfactored	1.45
Coefficient of Sliding	0.73
Seismic Coefficient (Kae)	0.28

Lateral Earth Pressure Coefficients and Soil Parameters

Piers 1 and 2, footing elevations at approx. elev. 1950'

Parameter	Value
Backfill Unit Weight (γ)	130 pcf
Backfill Soil Friction Angle (φ _f)	36°
Active Earth Pressure (K _a)	0.26
At Rest Earth Pressure (K ₀)	0.41
Bearing Soil Friction Angle (\$\phi_f\$)	30°
Passive Earth Pressure (K _p) - Unfactored	3.85
Coefficient of Sliding	0.58
Seismic Coefficient (K _{ae})	0.28

Shear Modulus versus Foundation Soil Strain

Piers 1 and 2

Shear Modulus, G	Strain	Poisson's ratio, μ
278 to 835 ksf	0.2 to 0.02 %	0.35

SR 395 Northbound Overcrossing at Parksmith Drive/BNSF

Pier 1Bearing Capacity

Maximum Footing Elevation: 1930'

Footing Width (ft)	Ultimate Bearing Capacity (ksf)	Service Bearing Capacity (ksf) Based on 1.5" of Settlement
6	60	19
9	69	13.5
12	76	10.5
15	83	9.0
18	91	7.5
21	99	7.0

Piers 2 and 3 Bearing Capacity

Pier 2 Maximum Footing Elevation: 1917'

Pier 3 Maximum Footing Elevation*: 1930'

* Requires over-excavation to elevation 1920' and backfilling with gravel borrow, and compacting in accordance with Method C compaction, to elevation 1930'.

Footing Width (ft)	Ultimate Bearing Capacity (ksf)	Service Bearing Capacity (ksf) Based on 1.5" of Settlement
6	25.0	11.5
9	30.0	8.25
12	35.0	6.75
15	40.0	6.0
18	45.0	5.25
21	50.0	4.75

SR 395 Northbound Overcrossing at Parksmith Drive/BNSF (cont.)

Lateral Earth Pressure Coefficients and Soil Parameters

Parameter	Value
Backfill Unit Weight (γ)	130 pcf
Backfill Soil Friction Angle (\$\phi_f\$)	36°
Active Earth Pressure (K _a)	0.26
At Rest Earth Pressure (K ₀)	0.41
Bearing Soil Friction Angle (\$\phi_f\$)	32°
Passive Earth Pressure (K _p) - Unfactored	1.45
Coefficient of Sliding	0.6
Seismic Coefficient (Kae)	0.28

Shear Modulus versus Foundation Soil Strain

Shear Modulus, G	Strain	Poisson's ratio, μ
610 to 1830 ksf	0.2 to 0.02 %	0.35

SR 395 Northbound Undercrossing at Perry Street

Pier 1 Bearing Capacity

Pier 1 Maximum Footing Elevation: 1875'

Footing Width (ft)	Ultimate Bearing Capacity (ksf)	Service Bearing Capacity (ksf) Based on 1" of Settlement
6	44.0	18.2
9	53.0	13.2
12	62.1	10.5
15	70.9	9.0
18	79.3	8.0
21	97.2	7.2

Pier 2 Bearing Capacity

Pier 2 Maximum Footing Elevation: 1865'

Footing Width (ft)	Ultimate Bearing Capacity (ksf)	Service Bearing Capacity (ksf) Based on 1" of Settlement
6	79.4	37.5
9	97.5	27.5
12	115.4	22.5
15	132.7	20.0
18	149.1	18.5
21	164.6	17.0

SR 395 Northbound Undercrossing at Perry Street (cont.)

Pier 3 Bearing Capacity

Pier 3 Maximum Footing Elevation: 1885'

Footing Width (ft)	Ultimate Bearing Capacity (ksf)	Service Bearing Capacity (ksf) Based on 1" of Settlement
6	21.2	21.2*
9	31.8	20.5
12	42.4	16.5
15	53.2	14.0
18	63.8	12.5
21	74.4	11.5

^{*} For the smaller footing sizes at Pier 3, the ultimate bearing capacity and service bearing capacity are the same. Settlements are estimated to be less than 1 inch.

Lateral Earth Pressure Coefficients and Soil Parameters

Parameter	Value
Backfill Unit Weight (γ)	130 pcf
Backfill Soil Friction Angle (φ _f)	36°
Active Earth Pressure (K _a)	0.26
At Rest Earth Pressure (K ₀)	0.41
Bearing Soil Friction Angle (φ _f)	36° at Pier 1, 40° at Piers 2 and 3
Passive Earth Pressure (K _p) - Unfactored	3.85 at Pier 1, 1.45 at Pier 2
Coefficient of Sliding	0.73 at Pier 1, 0.84 at Piers 2 and 3
Seismic Coefficient (Kae)	0.28

Shear Modulus versus Foundation Soil Strain

Pier 1

Shear Modulus, G	Strain	Poisson's ratio, μ
625 to 1875 ksf	0.2 to 0.02 %	0.35

Piers 2 and 3

Shear Modulus, G	Strain	Poisson's ratio, μ
823 to 2469 ksf	0.2 to 0.02 %	0.35